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# Impact and Repairs for End Region Cracking in Prestressed Girders

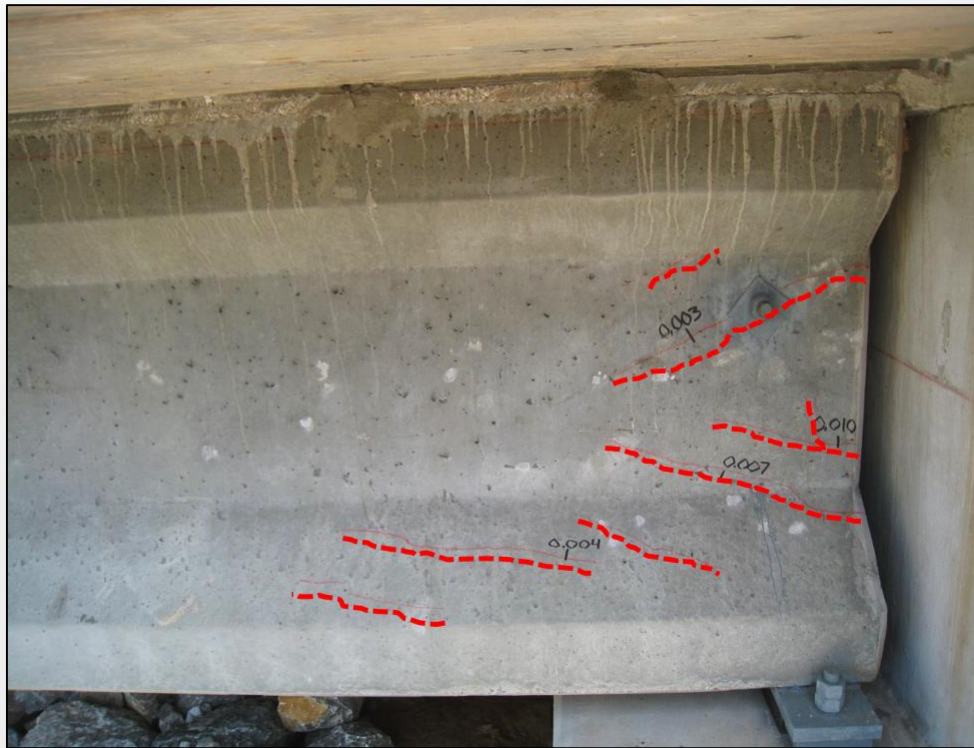
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# Problem

- Oklahoma DOT (ODOT) prestressed girders: frequent anchorage zone cracking



*Approx. 1-2 weeks after casting*



# Investigation for ODOT

- ✓ Phase 1: Review Existing Conditions
- ✓ Phase 2: Analyze Service Life & Repairs
- ✓ Phase 3: Design Crack Control Strategies
- ✓ Phase 4: Full-Scale Trial Implementation

# Field Investigation

- Girder cracking tracked from casting through deck addition at two bridges (2015 and 2017)



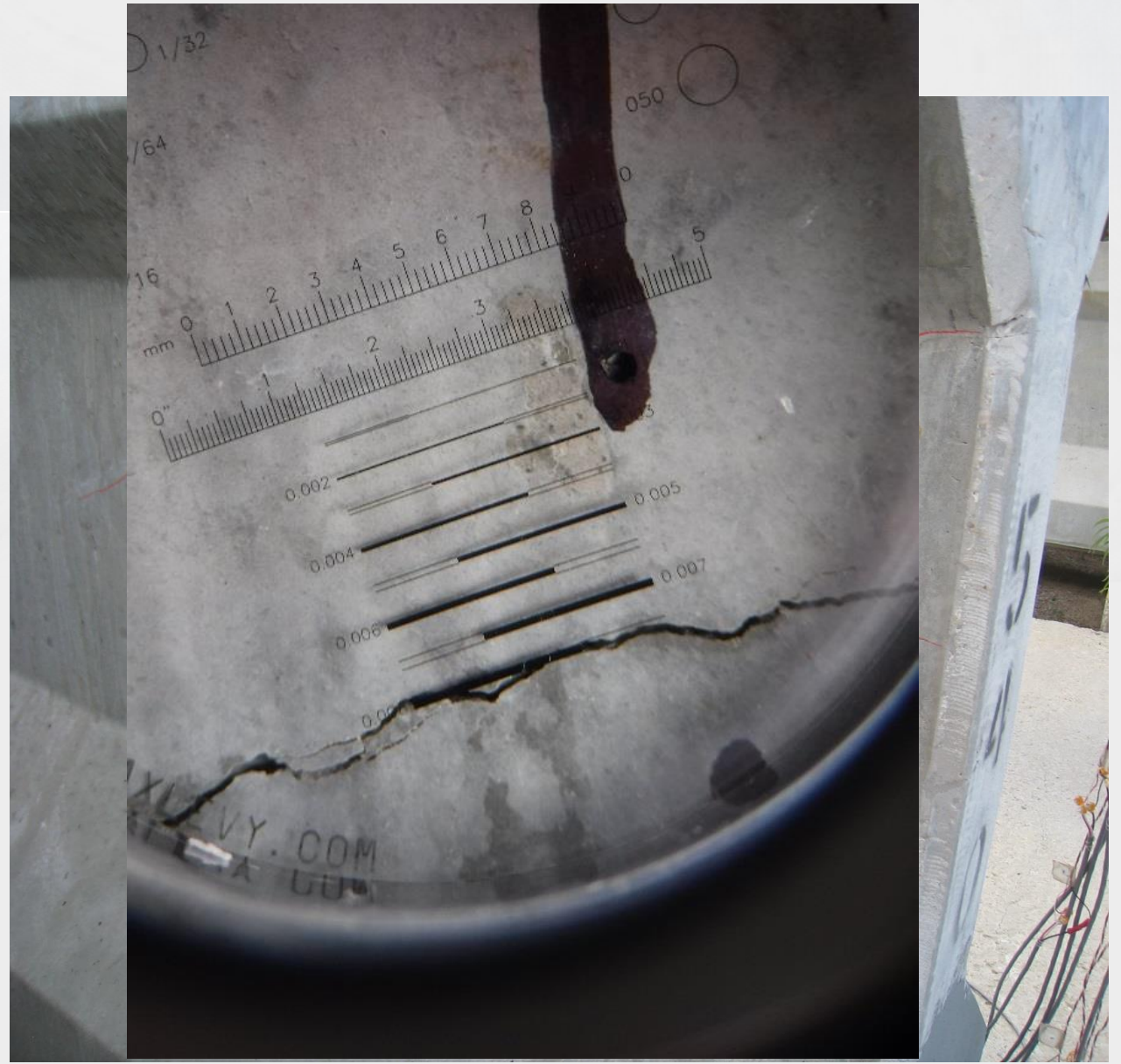
# Crack Distribution



- Crack density within first 40 inches:
  - 20 to 30 in. of crack per ft<sup>2</sup> of web area
  - Crack grew up to 10%-20% in first several weeks (3 to 6 in.)
- Cracks did not close when girders were erected or when deck was cast

# Crack Widths

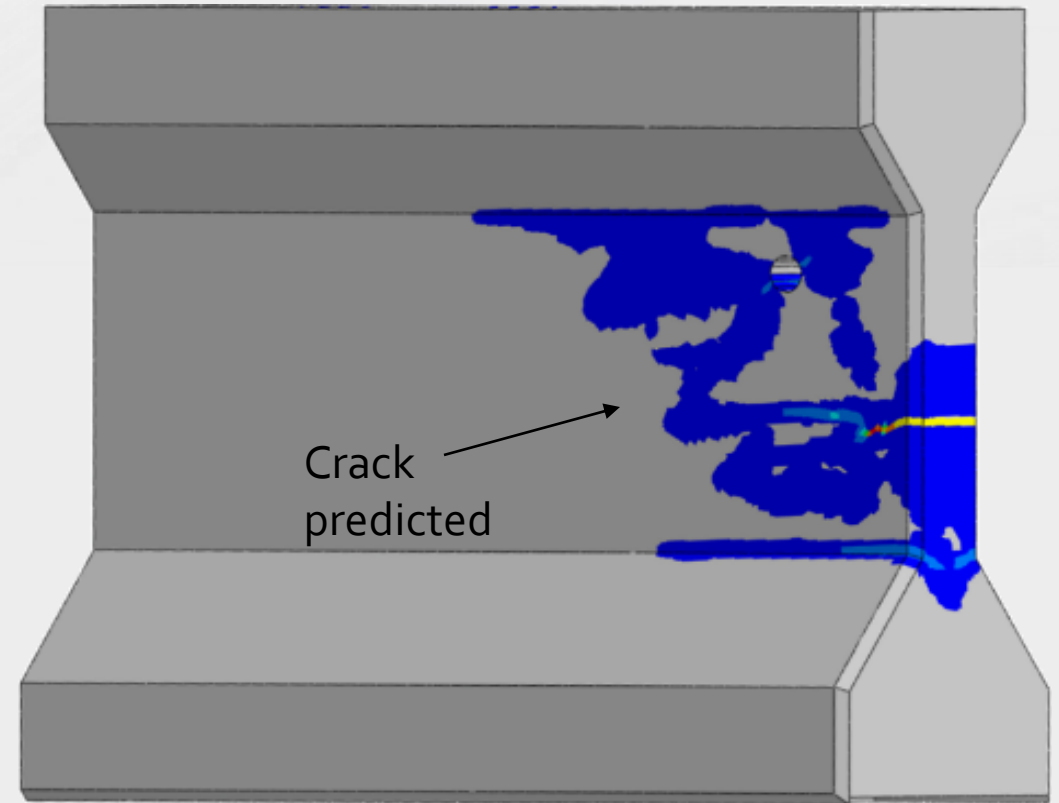
- Marked in field; measured using crack card and loupe
- Crack widths:
  - **8 to 10 mils**
  - Narrowing away from end



# Nature of Cracking

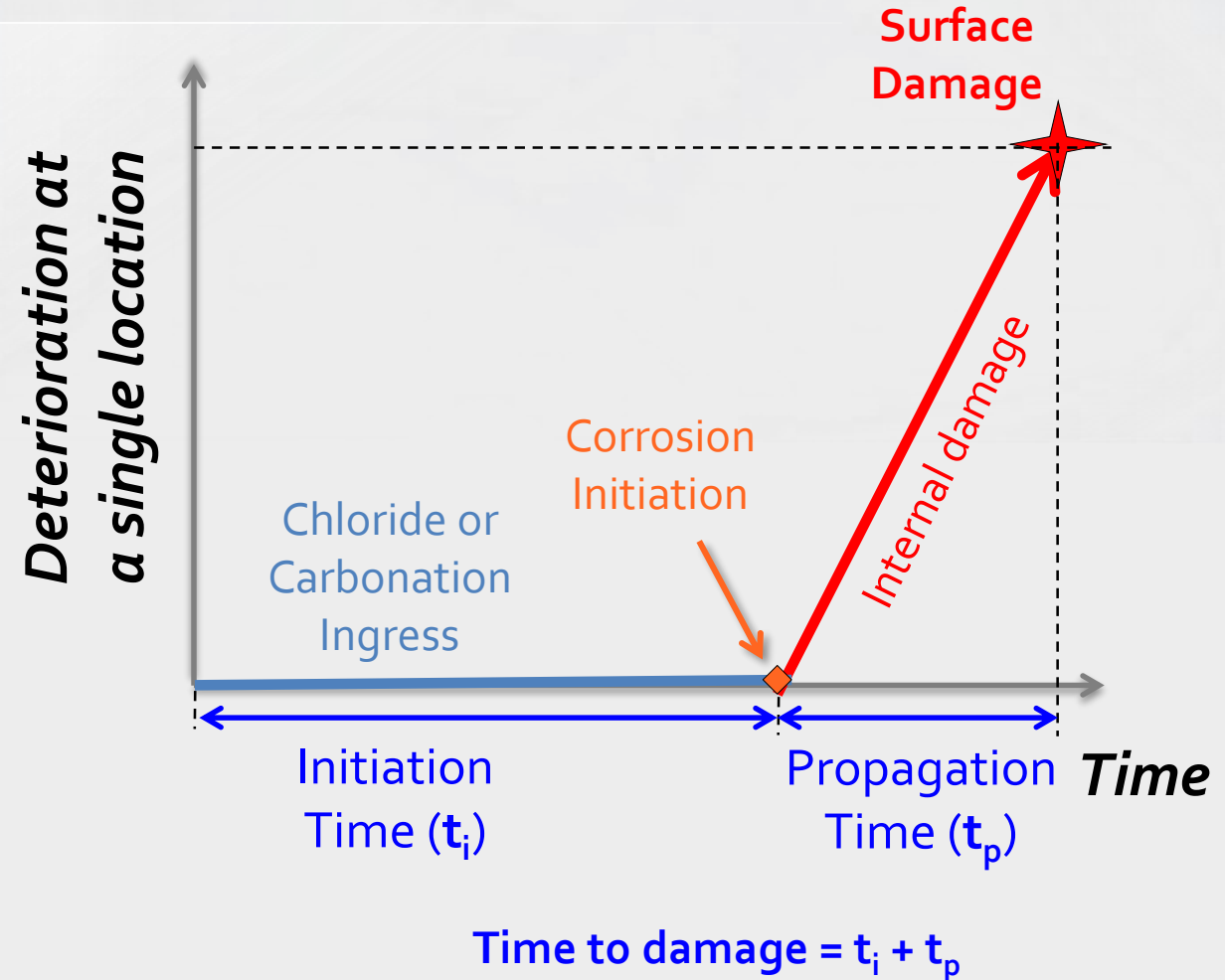
- High prestressing forces
  - Start at release or first lift; slight growth due to drying shrinkage
- Not structurally significant
  - Not aligned with strands; perpendicular to typical shear cracks

➔ Impact on Durability?



# Service Life Modeling with Cracks

- Model: Level of corrosion-related deterioration versus time
  - Chloride transport
    - Driving force: surface exposure
    - Mechanisms: diffusion, capillary adsorption, permeation
    - Resistance: concrete quality
- ➔ Reduced by Cracking



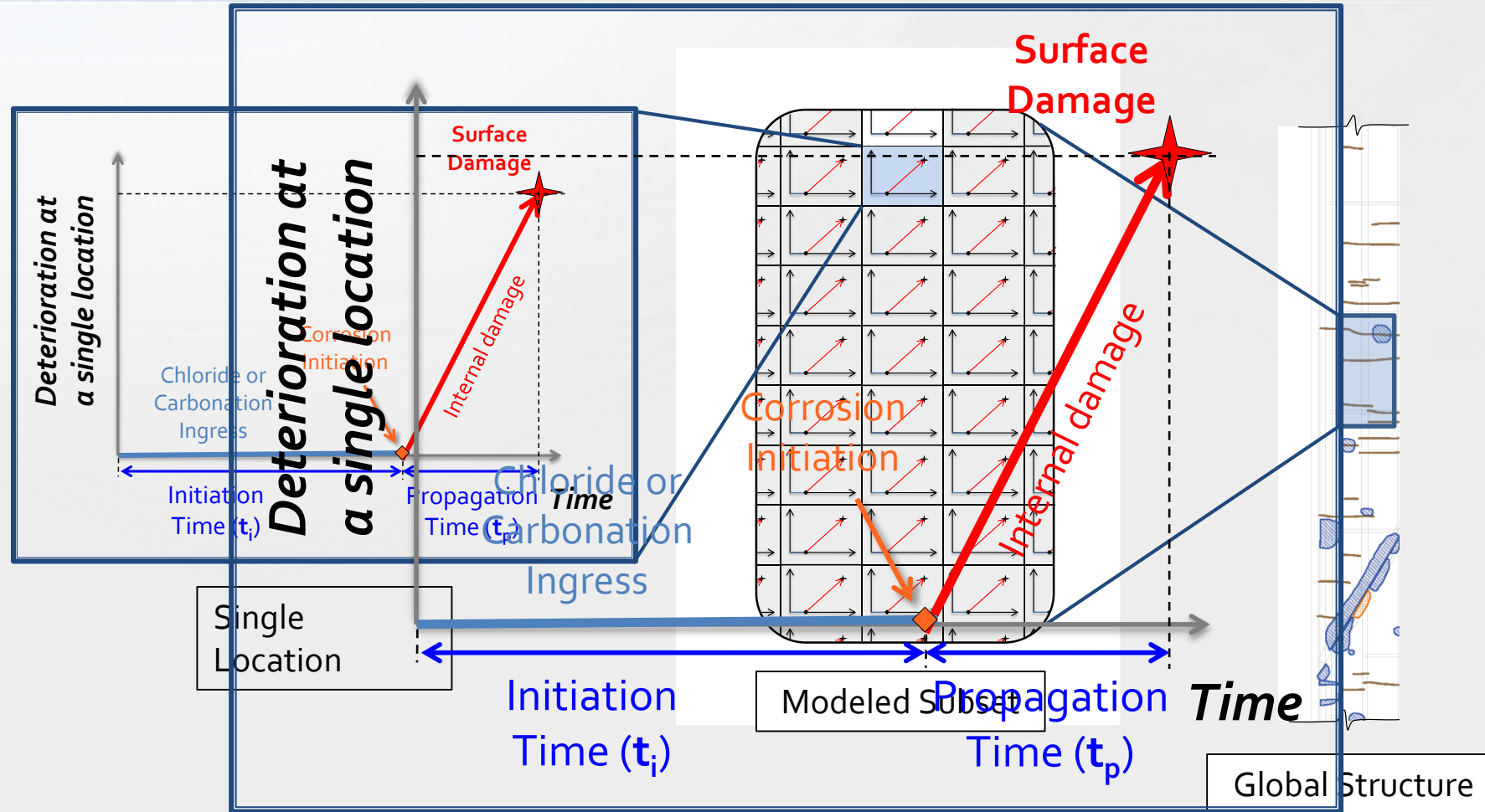


# Modeling Chloride-Induced Corrosion

- **End of Service Life =**  
**Deterioration > Limit**
  - Serviceability
  - Structural Integrity
- ODOT Girders: Corrosion-induced concrete surface damage > **15%**

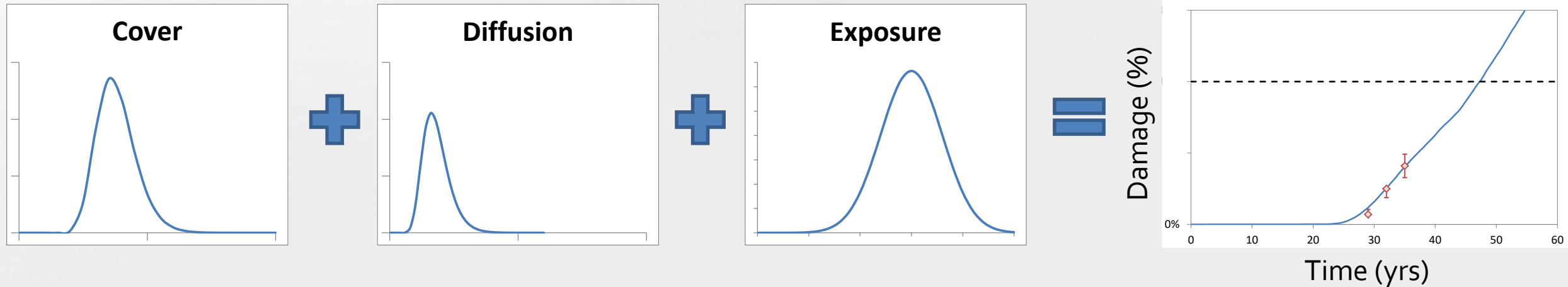


# Probabilistic Modeling

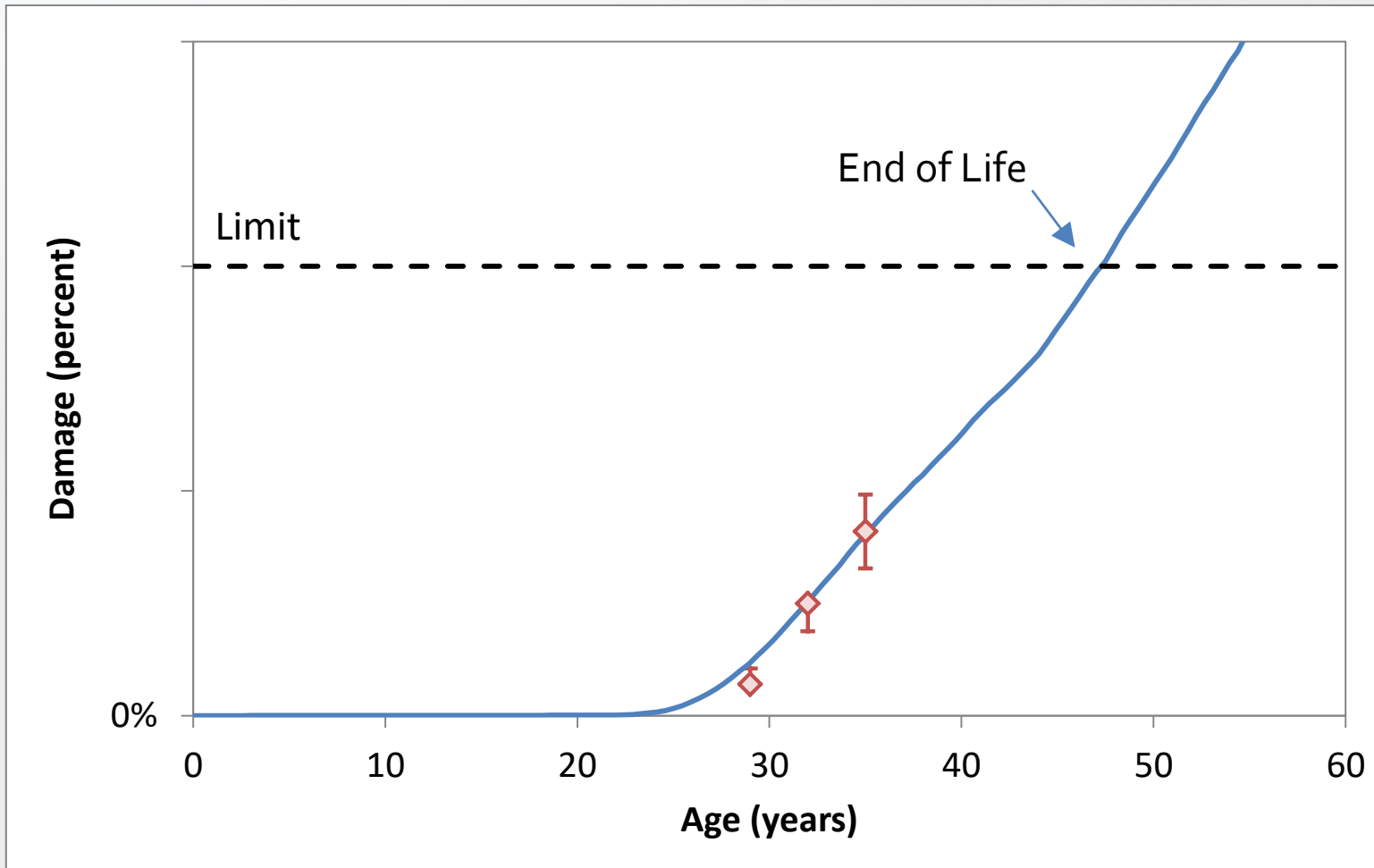


# Probabilistic Approach

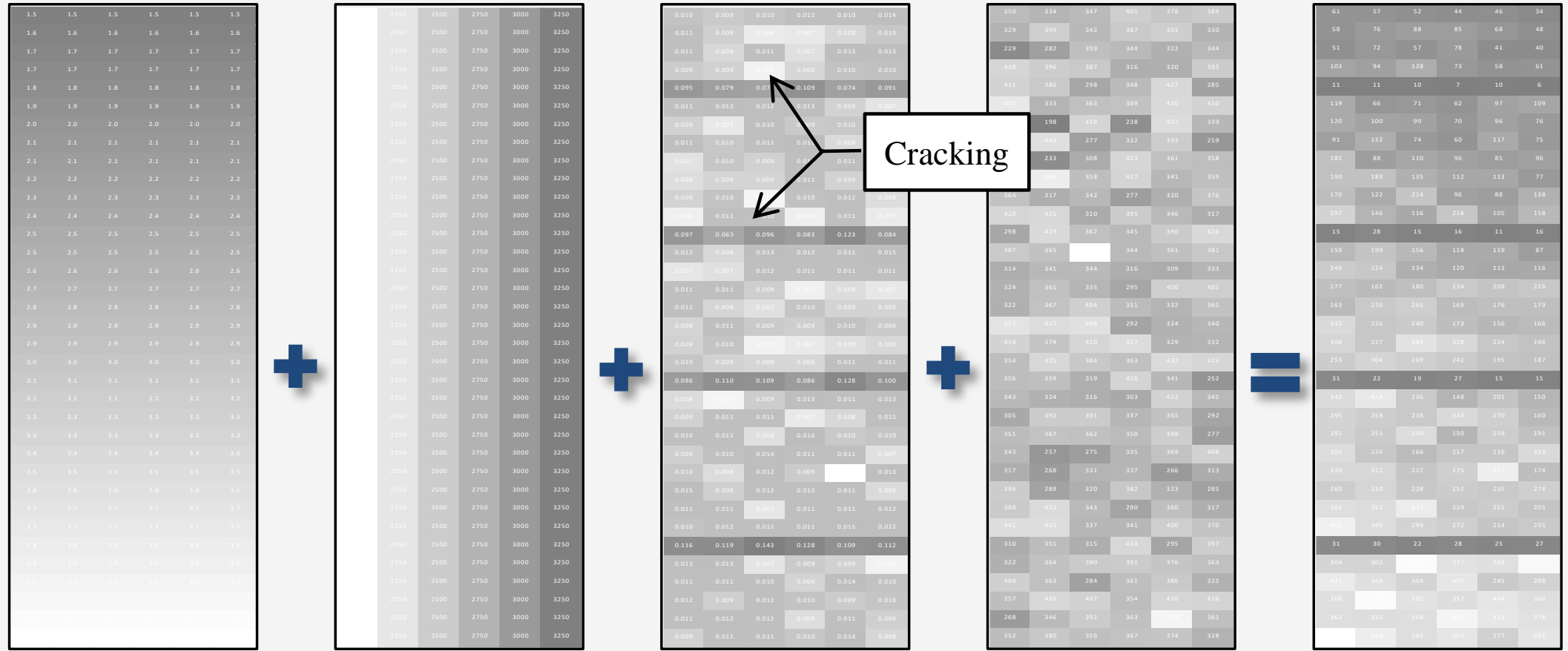
- Combinations of independent variables: Monte-Carlo Simulation



# Probabilistic Approach



# Probabilistic Analysis – Consideration of Cracks



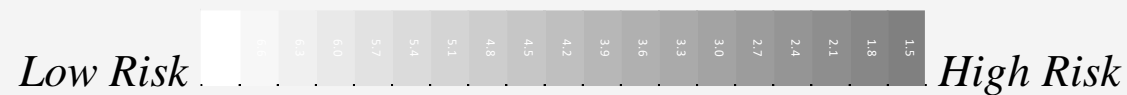
Cover  
(Input)

Exposure  
(Input)

Diffusion Rates  
(Input)

Chloride Threshold  
(Input)

Initiation Time  
(Output)



# ODOT Girder Chloride Exposure



- Chloride tests on several bridges
  - Variable by geography and traffic density; joint condition
  - Three general chloride exposure levels:
    - Low exposure = 1,000 PPM
    - Moderate exposure = 2,000 PPM
    - Severe exposure = 4,000 PPM

# ODOT Service Life Considerations

- Assumptions
  - Girder exposure
  - Concrete performance
  - Geometry (cover)
- Relative performance more important for ODOT
  - Allows cost-benefit evaluation of repair

# Parametric Study of Service Life

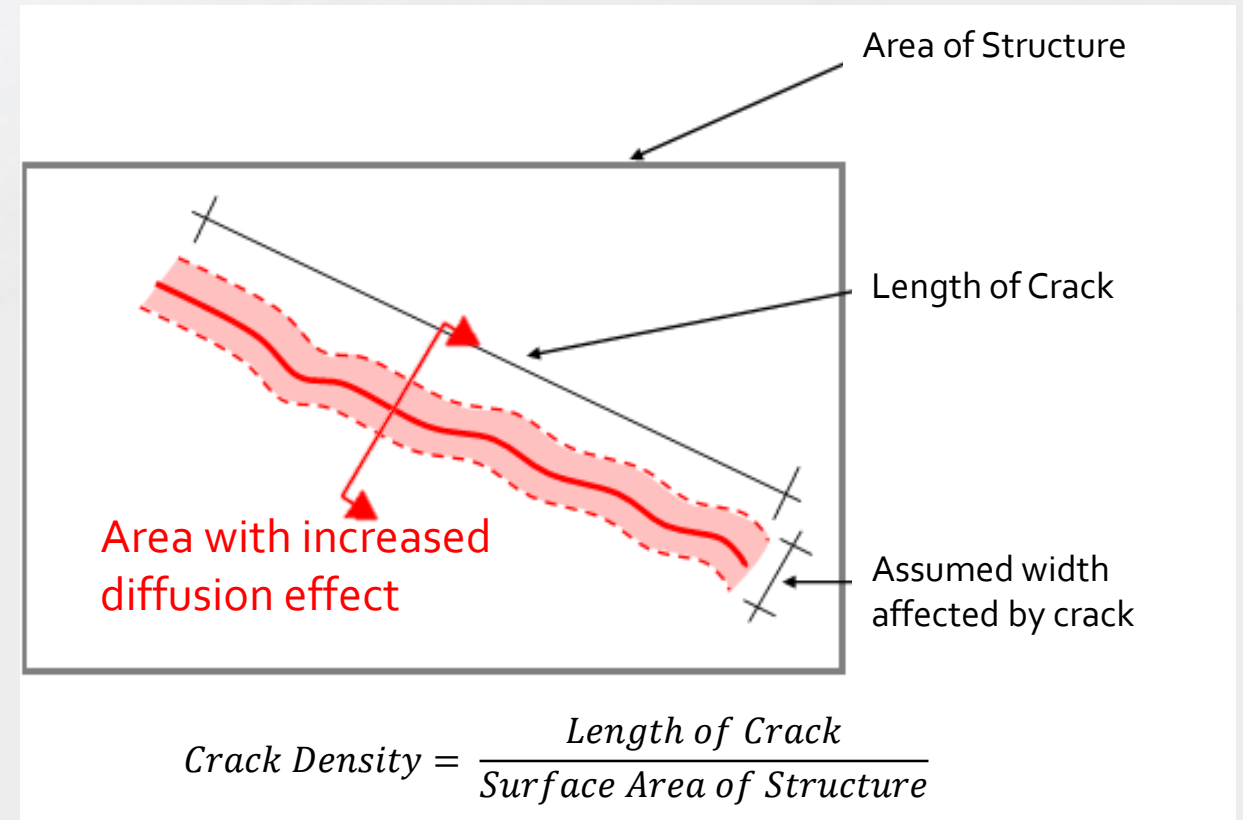
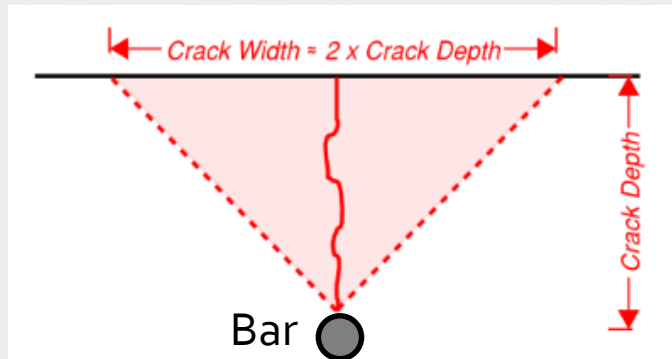
- Key factor: Cracking
- Factors:
  - Chloride exposure: Low, Moderate, or Severe
  - Crack density: 0 to 30 in<sup>2</sup>/ft
  - Crack width: 0 to 12 mils
  - Two mixtures (two precasters)





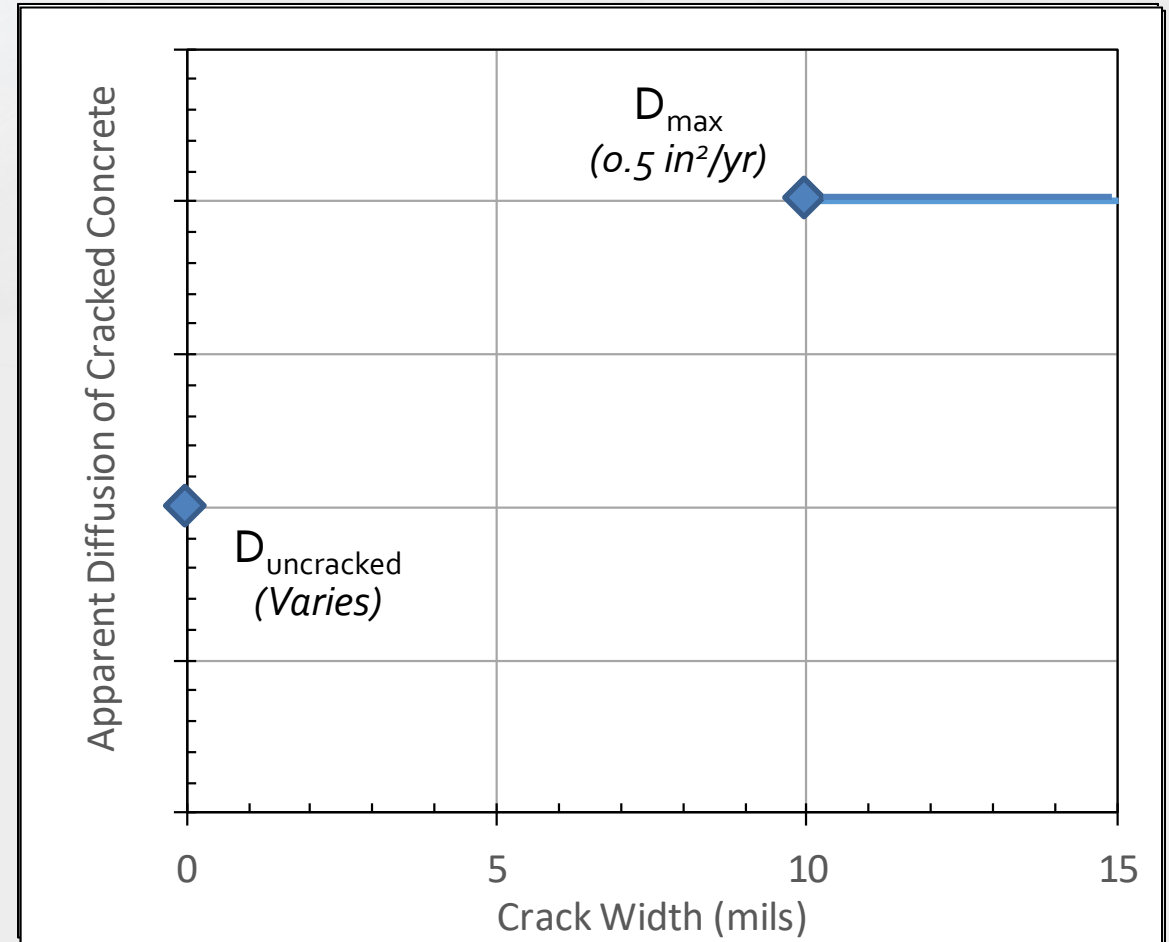
# Crack–Affected Surface Area

- Increased diffusion near cracks
  - Influence width equal to cover on each side of crack (45°)
  - ODOT: 1.5 in. concrete cover, so assume 3 in. of affected width



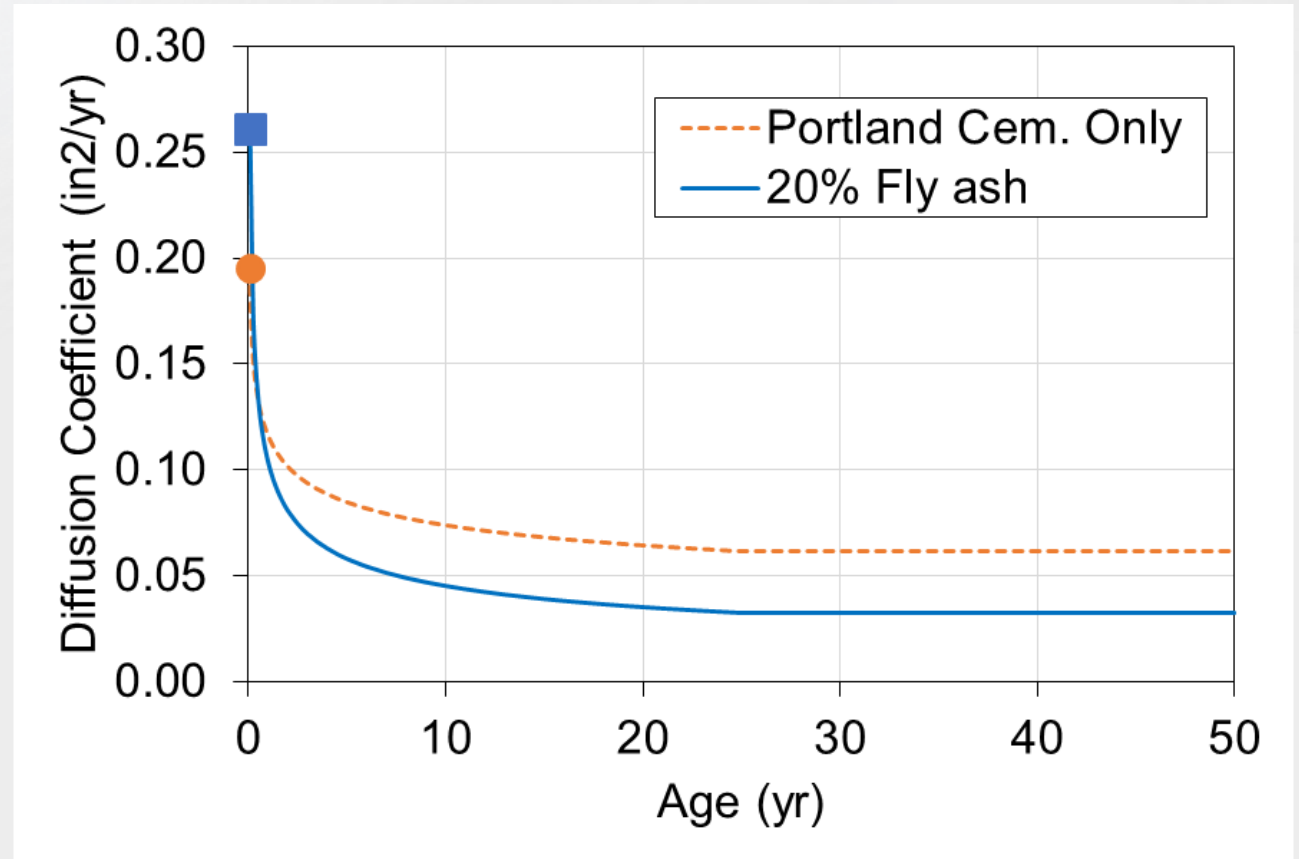
# “Diffusion Coefficient” for Cracked Concrete

- Uncracked diffusion based on concrete mixture
- Large cracks (>10 mils):  
Max. diffusion of 0.5 in<sup>2</sup>/yr

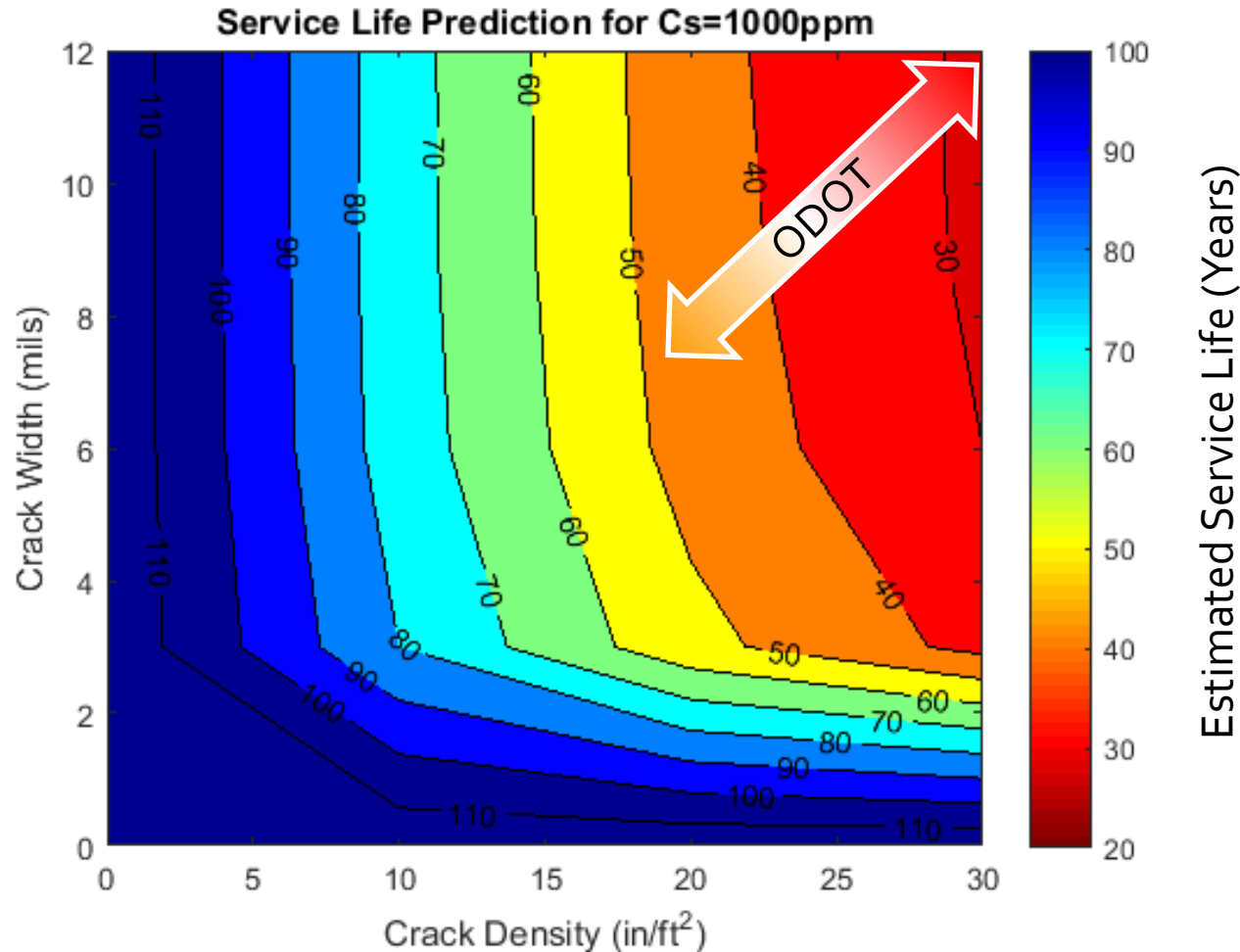


# Diffusion Coefficient for Uncracked Concrete

- Empirical Model for Diff. Coef.
  - Mix 1:
    - Portland cement only
    - $w/cm = 0.28$
  - Mix 2:
    - 20% Fly ash
    - $w/cm = 0.33$



# Predicted Service Life – Low Exposure

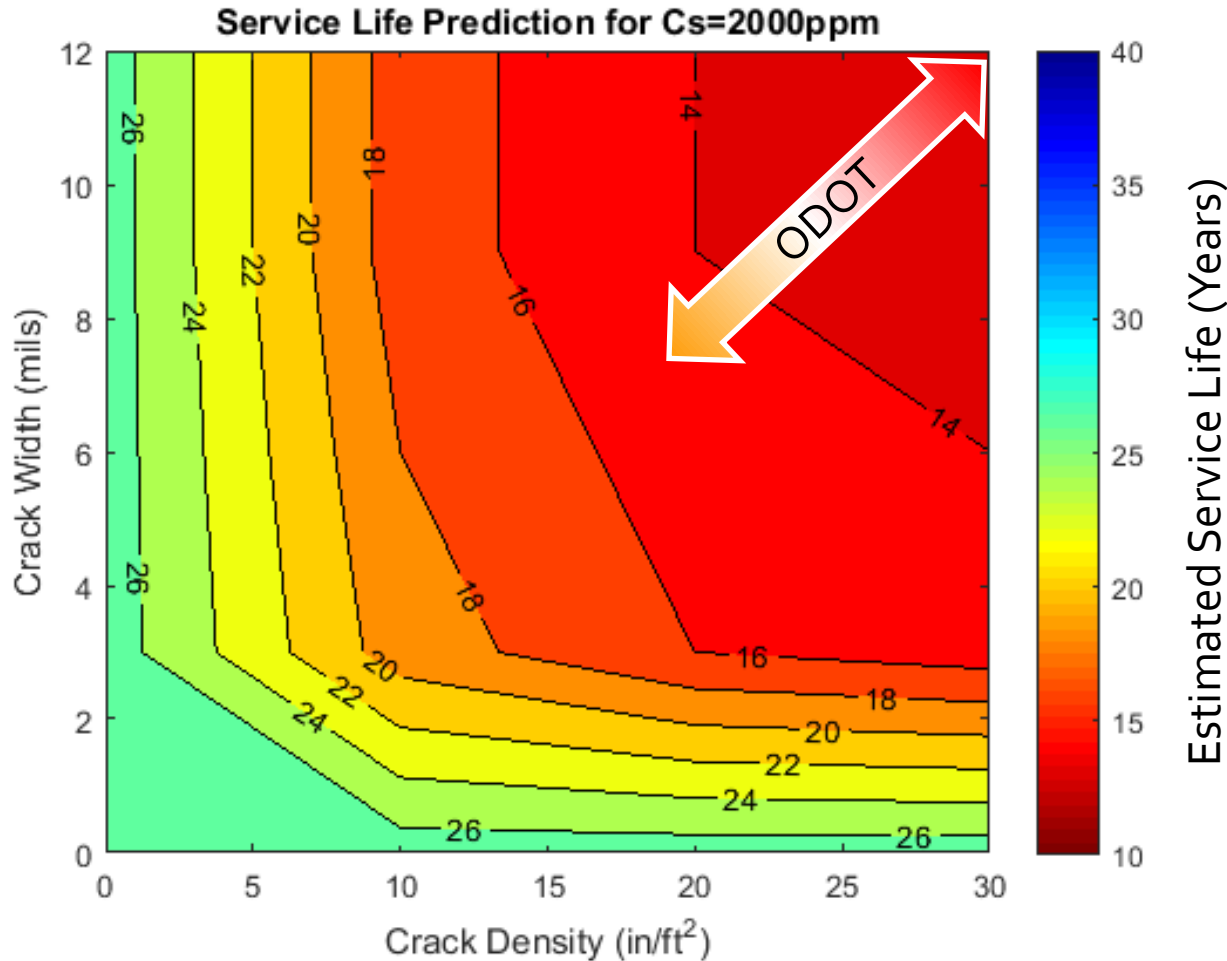


- SL reductions if both:
  - Crack density > 5 in/ft<sup>2</sup>
  - Crack width > 2 mils

## Predicted Life

Exposure	Uncracked	Cracked (observed)	Potential Reduction
Low	100+ yr	~30 yr	70%

# Predicted Service Life – Moderate (and Severe) Exposure



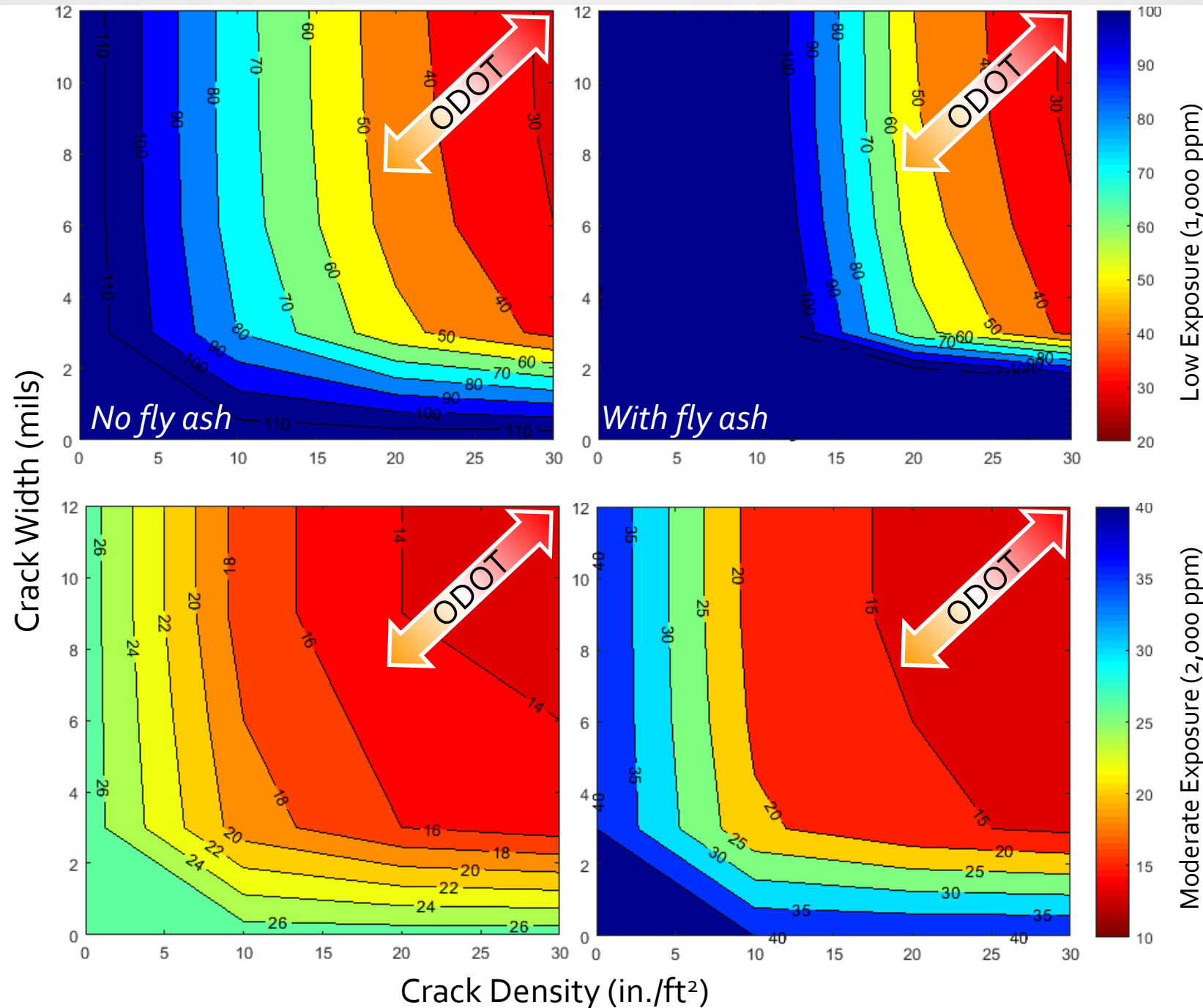
- SL reductions for nearly all crack width and density
- Relative magnitude of reduction less than low exposure

## Predicted Life

Exposure	Uncracked	Cracked (observed)	Potential Reduction
Moderate	~30 yr	~15 yr	50%
Severe	~17 yr	~10 yr	40%

# Influence of Mix

- In uncracked concrete, SCMs improved service life
- Cracks override benefit from SCMs



# Crack Repair

- **Service Life Modeling ➡ Cracks should be repaired**
- Conventional options:
  - Epoxy injection, but unnecessary for anchorage zone (not structural in nature)
  - Silane, but most efficient when addressing small cracks (< 10 mils)
- ODOT: No current consensus strategy

# Discrete Crack Seal

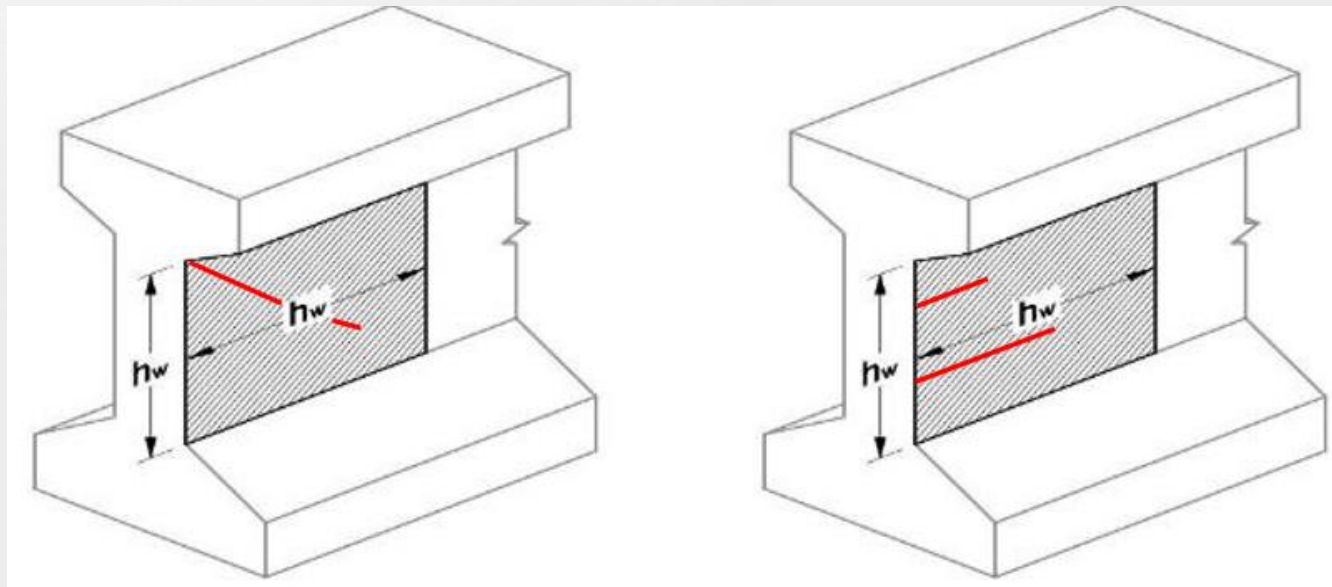
- For primary cracks (> 10 mils), local surface application:
  - Epoxy paste:
    - Rigid - trouble if cracks grow
  - Acrylic/silicone/polyurethane sealant:
    - Flexible - accommodate crack growth





# Blanket Crack Treatment

- More helpful if  $> 10 \text{ in./ft}^2$  of cracking present
- Review need after addressing wide discrete cracks



# Blanket Crack Treatment

- Epoxy/polymer-resin based surface sealers
- Silane with either acrylic paint or silicone resin coating
- High-build elastomeric polyurethane with aliphatic top coat

# Implementation of Combined Strategy

- Ensure compatibility of discrete and blanket treatment systems

System	Discrete Crack Seal	Blanket Treatment
<i>Rigid</i>	Epoxy	Epoxy/polymer-resin based surface sealers
<i>Flexible</i>	Acrylic/silicone/ polyurethane "caulk"	Silane & acrylic/silicone coating; or High-build polyurethane w/top coat

# Repair for New Girders



- Delay treatment until shortly before shipment or  $> 56$  days if possible, due to crack growth
  - Flexible materials more forgiving
- Recommend treat all girders

# Repair for Existing Girders

- Focus below expansion joints



# Conclusions

- High density of cracking in webs at anchorage zones not structural, but likely to impact durability
- WJE performed modeling considering cracks to quantify relative impact on service life
- Combined crack repair strategy to address both narrow and wide cracks recommended

# Thanks! Questions?



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