#### **Fighting Corrosion & Preserving Bridges**

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## **Corrosion-Related Concrete Damage**



#### Condition of Structure



#### Penn DOT - I-95 in Philadelphia



Corrosion in bridges leads to emergency closures and expensive repairs.





## Concrete Quality for 100-Year Life

- Concrete should have the following properties:
  - Strength, workability
  - Resistance to freeze thaw
  - Resistance to chloride penetration
  - Resistance to sulfate attack
  - Resistance to Alkali-Silica Reaction
  - Abrasion resistance



#### Time to Corrosion Initiation

#### **Diffusion Equation:**

$$C_{x,t} = C_o \left[ 1 - erf\left(\frac{x}{2\sqrt{Dt}}\right) \right]$$

#### • Using

- > Age (years)
- Rebar Cover of 1.5", 2.5", and 3.5"
- Average surface chloride for deck, substructure, and piles
  - in marine environments
- Chloride at the rebar = 400 ppm
- Diffusion coefficients (in²/yr.) of:
  - ✓ 0.01 in<sup>2</sup>/yr. Excellent durability,
  - ✓ 0.03 in<sup>2</sup>/yr. Good to fair durability,
  - ✓ 0.09 in<sup>2</sup>/yr. Poor Durability



#### Time to Concrete Damage for Various Rebar Depth



Diffusion property and cover varies within a bridge



## **Chloride-Induced Corrosion**

- Chloride from deicing salt application diffuses into concrete
- When chloride at rebar level exceeds 1.2 lb/CY, passive film breaks down and corrosion initiates
- If pH <11, corrosion can initiate at lower chloride levels
- If sulfate is present, chloride may not be required for corrosion to begin



## **Diagnosis before Treatment**

- When a bridge experiences corrosion, we want to answer the questions:
  - How bad is bad?
  - What is the rate of deterioration?
  - How do we cost effectively extend the life?
- SCS develops a strategic inspection/evaluation plan to quickly indentify/quantify problems.
- Average preservation cost for owners:
  20 to 25% compared to replacement.



#### **Assessment of Concrete Structures**

- 1. Non-Destructive Evaluation (earlier identification)
  - Identify and quantify deterioration of concrete and steel
- 2. Electrochemical Testing
  - Quantify time-to-failure, corrosion rates, future section losses
- 3. Laboratory Testing
  - Additional material and corrosion analysis
- 4. Estimate Service Life
  - Recommend cost effective solution



## **Non-Destructive Testing (NDT)**

- Use NDT to see hidden problems
- Minimize inspection time and damage to the structure
- Primary NDT tools:
  - Ground Penetrating Radar (GPR)
  - Infrared Thermography
  - Impact-Echo
  - Ultrasonic Tomography



## Laboratory Testing

- Laboratory Testing
  - Chloride Content Profiling (AASHTO T-260, ASTM C1152)
  - Chloride Migration Test NS State (NT Build 492)
  - Apparent diffusion coefficient (ASTM C1556, NT BUILD 443)
  - pH Indicator (Phenolphthalein)
  - Rapid Chloride Permeability (ASTM 1202)
  - Compressive Strength (ASTM C39)
  - Petrographic Analysis to Examine:
    - General Concrete Properties (density, air-void, w/cm) (ASTM C876)
    - Alkali-Silica Reactivity
    - Freeze-Thaw Damage (ASTM C472)



## Sampling Size

• Chloride cores shall be 4-inch diameter



A smaller core or powder samples can lead to significant variation in chloride level .

More sampling locations needed



## Processing Chloride Cores

- Mark 0.5-inch horizons along the depth of the core.
- Dry cut through the core at each horizon into concrete discs (slices).
- Pre-crush each slice into ~0.25-inch maximum size pieces.
- Pulverize each pre-crushed slice and pass through #50 sieve.
- Thoroughly clean after each pre-crush and pulverize session.
- Digest each sample in acid to extract chloride from the concrete powder.
- Titrate each sample to determine the chloride content.
- Process titration data to obtain chloride content.
- Perform chloride test at various depths of the core to obtain chloride profile for each core.
- Tabulate chloride data at various depths for analysis and service life calculations.



#### Case Study 1 I -581 over Williamson Road, Roanoke, VA





## **Bridge Information**

- Built: 1968
- Regular reinforced concrete
- 5 Spans, 4 piers, 2 abutments



#### **Visual Conditions**













### SCS Approaches

- Visual survey
- Delamination survey
- Concrete cover
- Chloride profile analysis
- Carbonation
- Petrographic analysis
- Service life modeling



## **Inspection Findings**

Element	% Damage	Avg. Cover (in)	95% Cover (in)	Cl% over 1000 ppm	Cl% over 500 ppm	Avg. Diffusion Coeff. (in²/yr)	Carbonation Depth (in)	Petro. Analysis
Pier Caps	25.3	2.06	1.01	60%	60%	0.070	0.50	Generally good quality concrete
Pier Columns	17.3	2.50	1.48	17%	17%	0.018	1.15	
Abutments	4.2	2.67	1.15	25%	25%	0.039	0.64	



#### **Service Life Analysis**

• Using chloride profile, cover, and concrete damage, develop time to corrosion initiation and future concrete damage.



#### Service Life Processing – Pier Caps





#### Service Life Processing – Pier Columns





#### Service Life Processing - Abutments





#### **Conclusions and Viable Options - Piers**

- Viable repair options:
  - A. Patch repairs + Impressed Current Cathodic
    Protection (ICCP)
  - B. Patch repairs + Electrochemical Chloride
    Extraction (ECE) + a breathable sealer, or
  - C. Patch repairs + sprayed Galvanic Cathodic
    Protection (GCP) system



## **Conclusions and Viable Options - Abuts**

• The viable repair options:

- A. Patch repairs + discrete GCP anodes + seal
- B. Patch repairs + thermal sprayed GCP, or
- C. Patch repairs + ECE + a breathable sealer



#### Life Cycle Cost Estimate

Bridge Element	Description	Initial Cost	Additional Repair Cost (50 years)	Additional MOT Cost (50 years)	Total
		\$784,849	\$147,311	\$0	\$932,160
Pier Caps	Patch + ECE				
Pier Columns	Patch + ECE + Seal	\$231,000	\$85,633	\$18,206	\$334,839
	Patch + ICCP	<mark>\$229,032</mark>	<mark>\$147,311</mark>	<mark>\$0</mark>	<mark>\$376,343</mark>
Abutments	Patch + Anodes +	\$12,589	\$49,250	\$0	\$61,840
	Seal				
Su	btotals	\$1,028,438	\$282,194	\$18,206	\$1, 328,839



#### **SCS** Recommendations

- Pier Caps Patch + ECE + Seal
- Pier columns Patch + ECE + Seal
- Abutments Patch + Discrete Anode + Seal



## Limitations of ECE

- ECE is not suitable for structures with high strength steel
- ECE is not suitable for structures with moderate to severe ASR



#### ECE AND SPRAYED ZINC ANODE ON 11 BRIDGES IN RICHMOND, VA



#### ECE on Pier – 11 Bridges





#### ECE on Pier – 11 Bridges





#### Thank you!

# Questions

