

2017 SEBPP ANNUAL MEETING

# Preserving the New River Gorge Bridge

Chad Robinson, PE – WVDOT

Matt Lewellyn, PE – B&N



**BURGESS & NIPLE**

Engineers ■ Architects ■ Planners



**TSP 2**  
AASH|O

TRANSPORTATION SYSTEM PRESERVATION  
TECHNICAL SERVICES PROGRAM

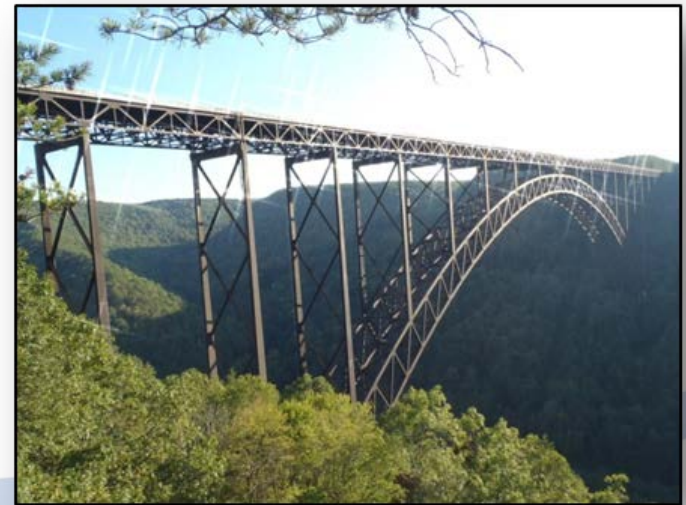
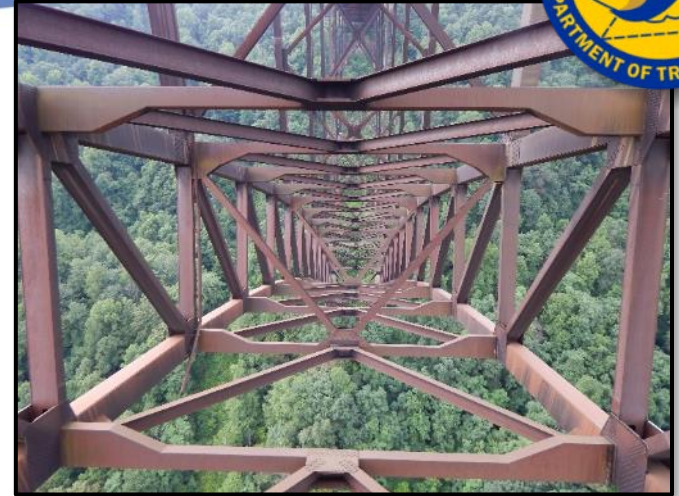
**BRIDGE PRESERVATION**

Tuesday, April 11, 2017 11:20-12:00 AM

# Presentation Overview



- ❖ Overview
  - ❖ Recent Projects
  - ❖ Bridge Configuration
- ❖ Project Specifics
  - ❖ Inspection Video
  - ❖ Load Rating
  - ❖ Preservation Techniques
  - ❖ Bearing Slide
- ❖ Questions and Answers





# “The” New River Gorge Bridge



June 1974 – October 1977  
\$37 Million

- ❑ Main Span Length: 1,700 ft Arched Truss
- ❑ Overall Length: 3,031 ft
- ❑ Height above the New River: 876 ft
- ❑ Unique Claim: Longest Steel Arch Bridge in the Western Hemisphere
- ❑ Added to the National Register for Historic Structures in 2013
- ❑ Today's Cost:

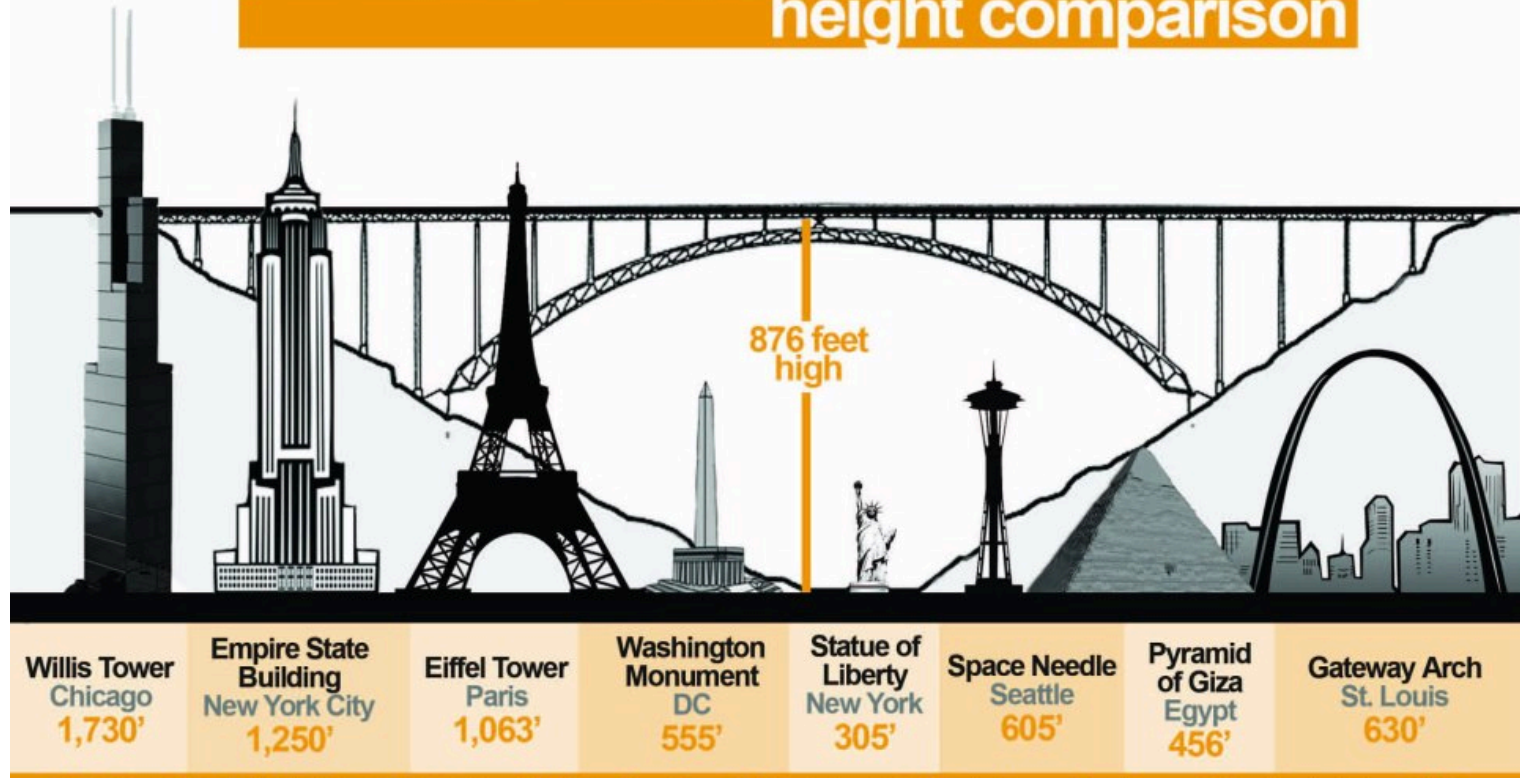
Today \$300-350 Million



# How High is the New River Gorge Bridge?



## NEW RIVER GORGE BRIDGE height comparison



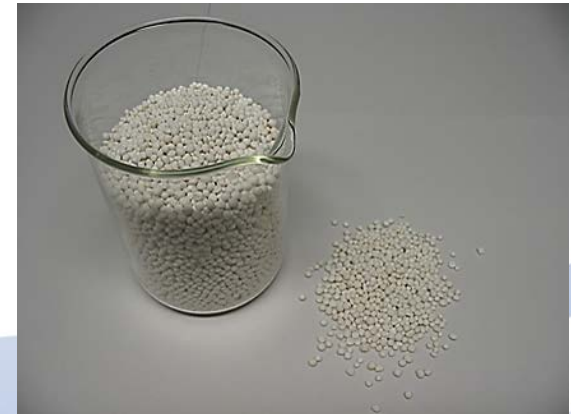
Si Du River Bridge (World's Highest Bridge) China 1,550' | Royal Gorge Bridge Colorado 1,053' | Hoover Dam Bypass Bridge Nevada/Arizona 900'



# Weathering Steel and De-icing



- Salt Spray from 1977 to 1998
  - Corrosion Initiated
  - Leaking into Box Members
- 1998 Maintenance
  - Washing to Remove Salt
  - Replaced Deteriorated Bolts
  - Caulked Box Members
- Current De-icing Plan
  - Calcium Magnesium Acetate
  - Salt stops on approaches



# Recent Projects on the Gorge



- 2010 – Deck Overlay
- 2012 Rehab ~ \$3 Million
- 2008-2014 Inspections
  - Load Rating
  - Rehabilitation Plans
- 2015-2020 Inspections
  - Element Level
- 2016 Rehabilitation
  - 4 Bidders
  - \$4.3 Million to \$6.3 Million

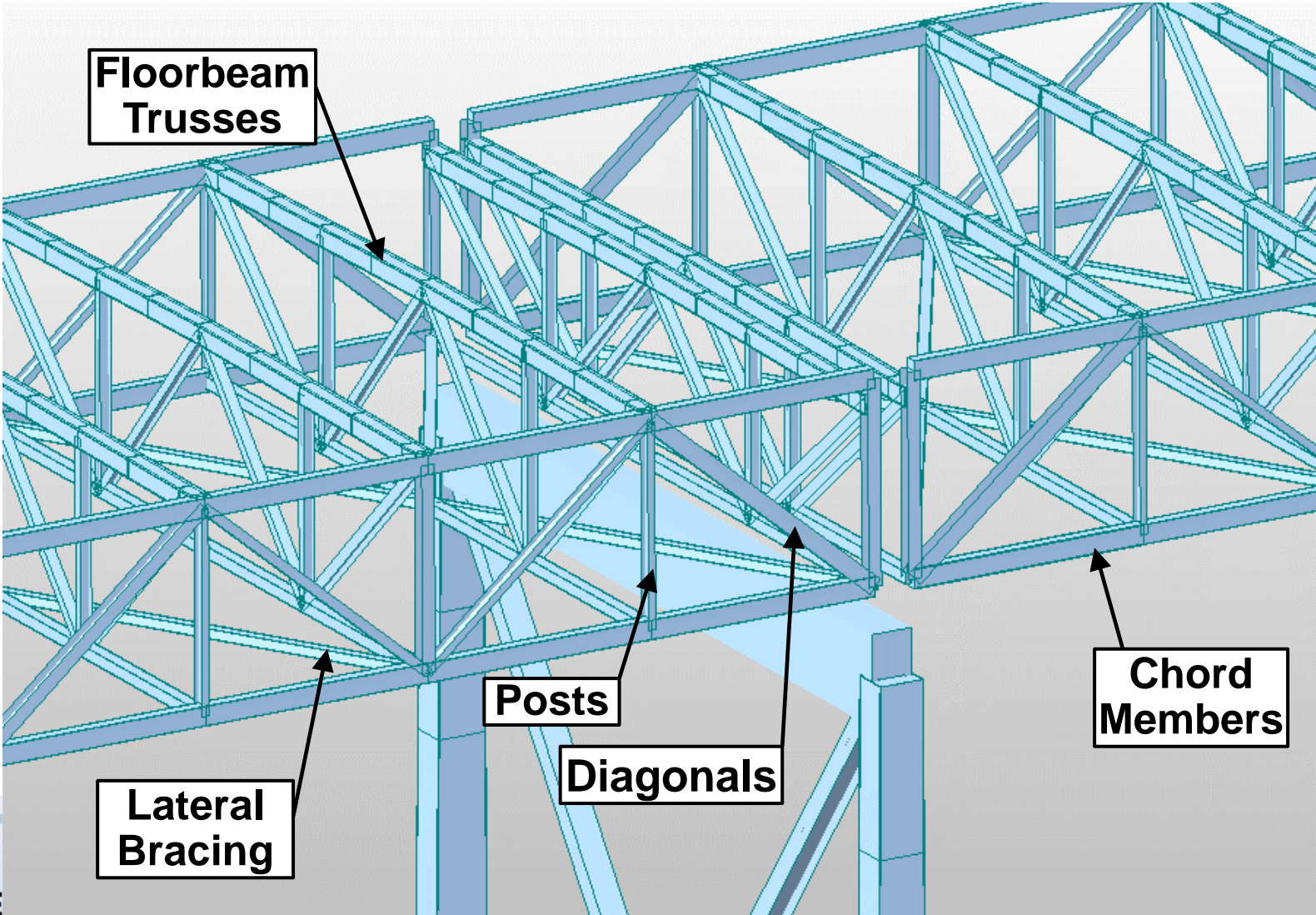


**BURGESS & NIPLÉ**  
Engineers ■ Architects ■ Planners

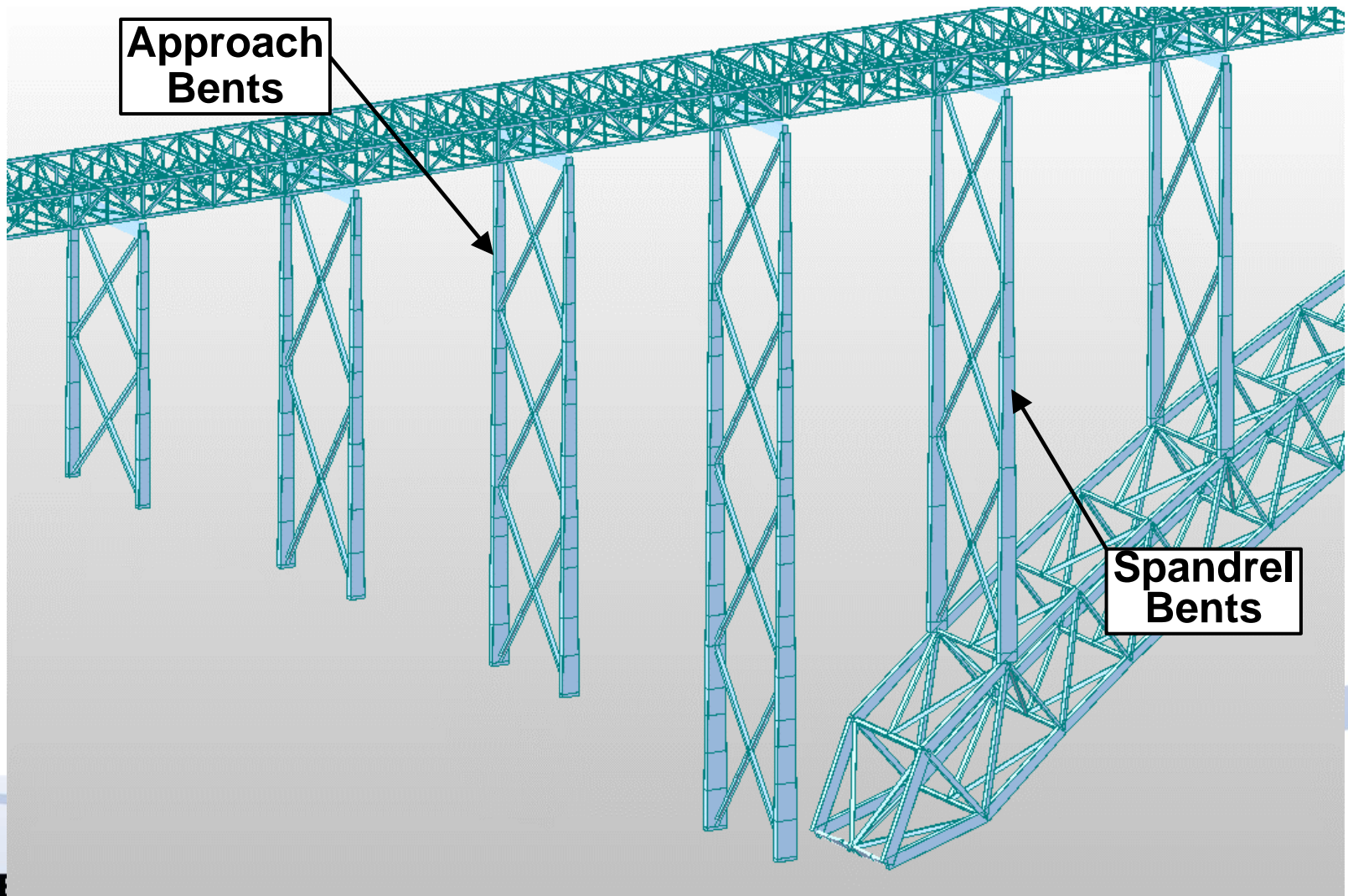




# Bridge Configuration - Deck Truss

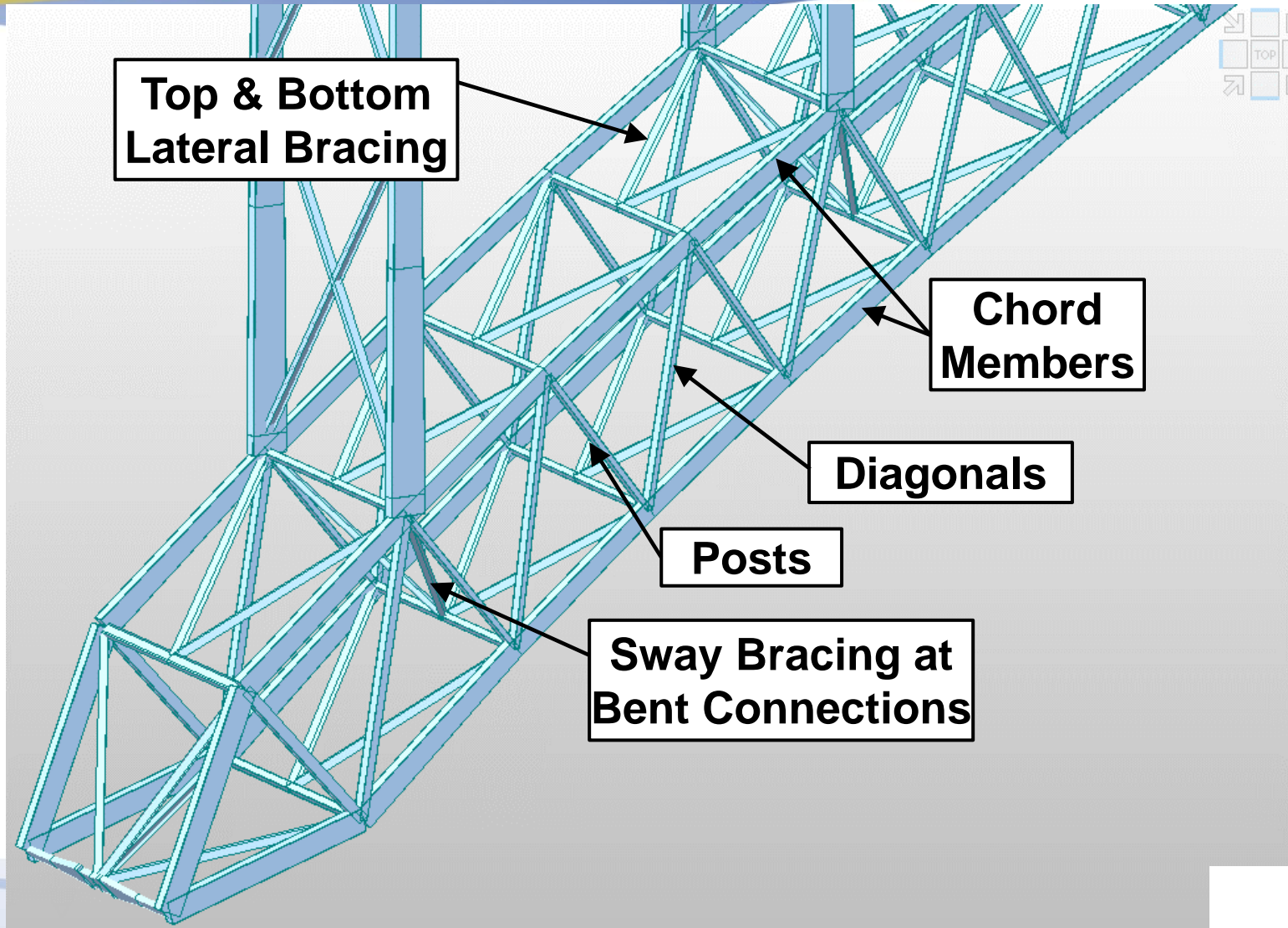


# Bridge Configuration - Bents





# Bridge Configuration - Arch Truss



# Bridge Inspection Access – Interview with B&N Inspectors



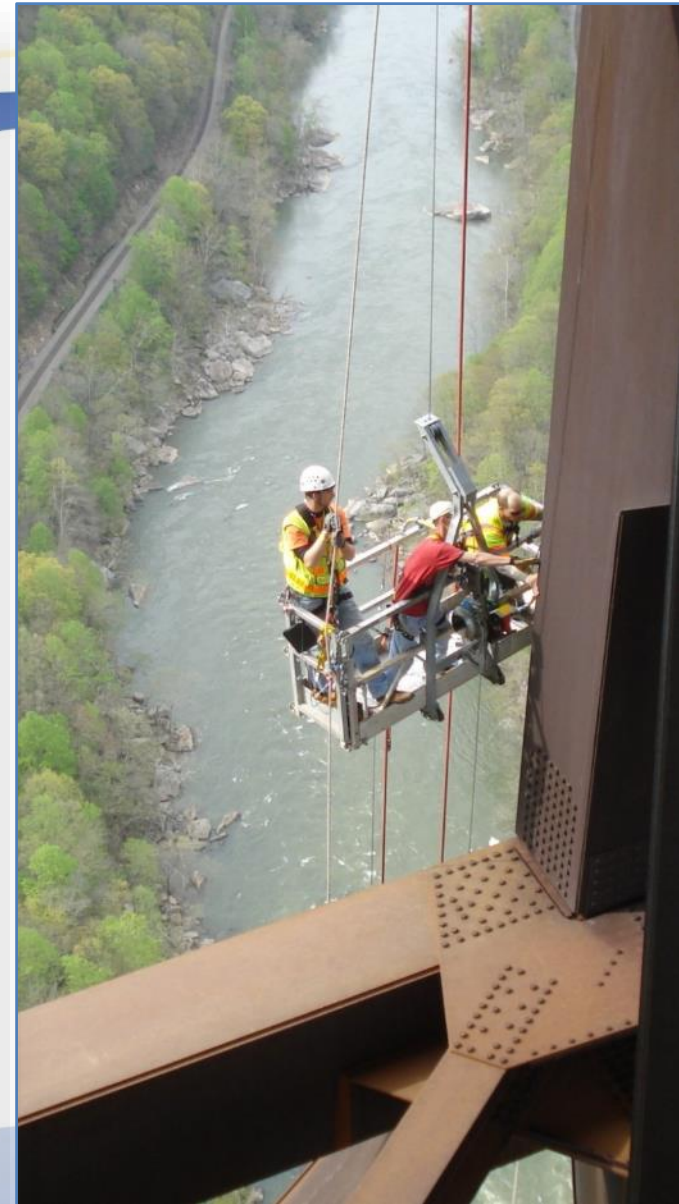
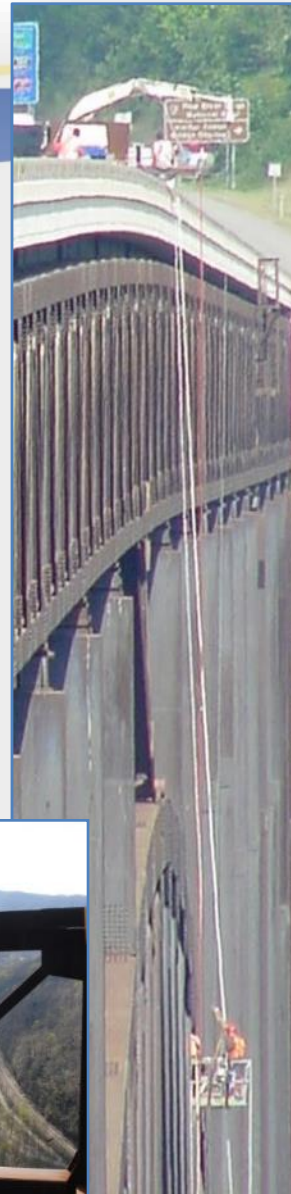
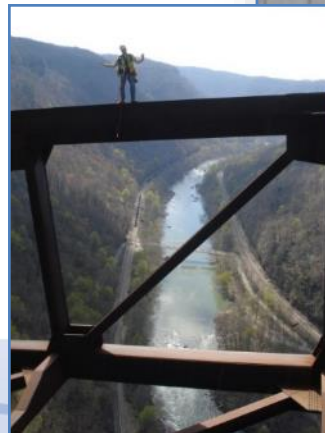
Prepared by Maximus Films Production for the German  
Documentary on the Worlds Most Extreme Bridges



# Arch Access

- ❖ Spider Access
  - Truck Crane
  - Cable
  - Skyclimber
  - Spider Basket
  - Safety Ropes

- ❖ Rope Access



# Unique Tidbits of Information

## ❖ Falcons

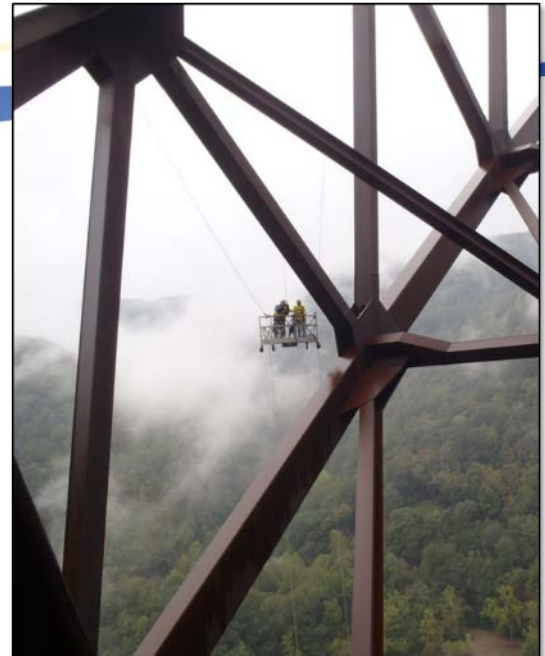
- Nesting at Bent 13 lower lateral bracing



## ❖ Bi-planes and FAA

- Keep your ropes short

## ❖ *BridgeWalk* Tours



*BridgeWalk* Tours





# Understanding Long Term Issues – Deck Truss

- ❖ Weathering Steel
- ❖ Prolonged Wetting
- ❖ Poor Bolt Sealing



**Pigeon Nesting**

**Nut Loss**



**Pack Rust**



**Section Loss**



# Understanding the 2012 Rehab



- Vacuum Clean
- Pigeon Waste
- Water Blast
- Caulk Seams
- Apply Penetrant Sealer
  - Inside handholes
  - Areas of corrosion
- Bolt Replacements



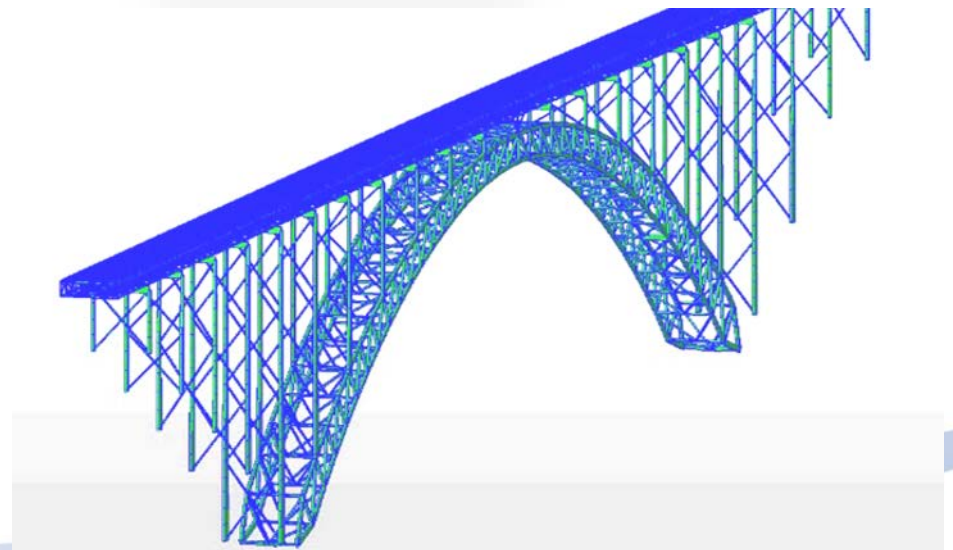
# How do we keep them out!?!

- ❖ WVDOT Needed a Reasonable Solution
- ❖ **Patented Magnetic Bird Screens**
- ❖ Installed on New River Gorge Bridge from 2008-2015
- ❖ On-going Monitoring of Field Performance
- ❖ Continue to Develop



# Complexity of Structural Analysis

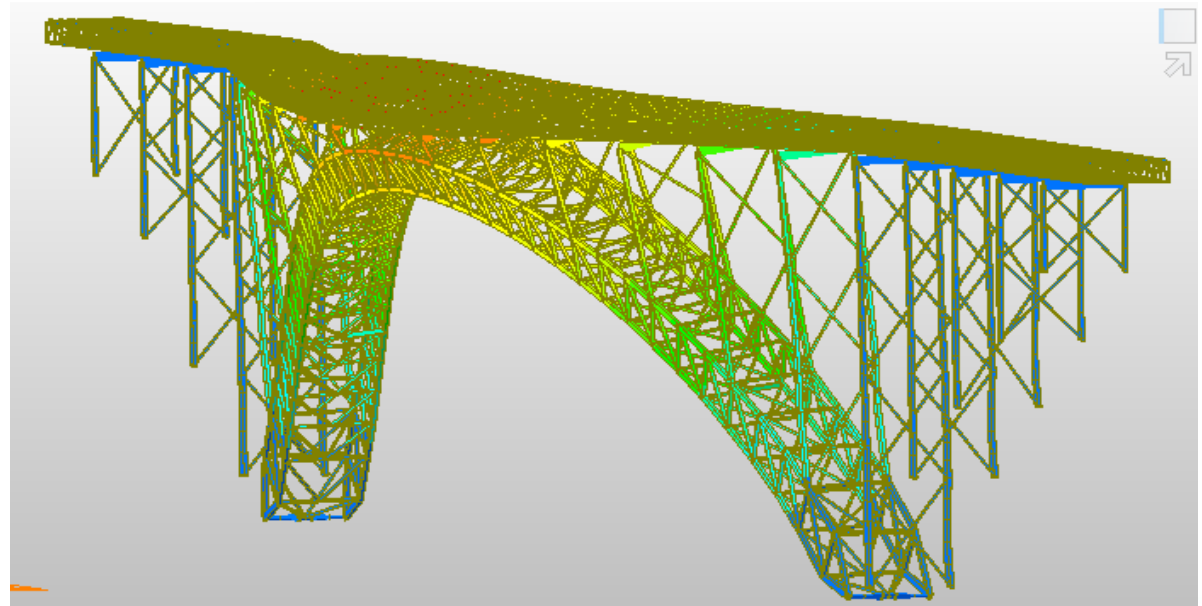
- ✓ 3-D Finite Element Model
- ✓ 15 Rolls of Shop Drawings
- ✓ 4,840 Rated Members
  - ✓ Wind Analysis
  - ✓ Thermal Forces
  - ✓ CRTS Loads
- ✓ Complex Gusset Plates
- ✓ 1 ½ Man Years of Work





# Nonlinear Wind Analysis

- Modeled using Midas Civil Software.
- Geometric nonlinear analysis was conducted for 12 wind combinations + dead load.



# Data Collection

- Member section loss data from bridge inspection reports.
- Location and depth of loss area.
- Automated real-time graphic display of the loss.

Section Loss					Loss Data							Calc	Check	
Elem. No.	Member Name	Loc	Type	end-I	Sect.	Comp.	Face	At Opening?	Reference Point	Offset	t	b		
11101	L1-L2	Truss 1L	LC	end-I										
11101	L1-L2	Truss 1L	LC	end-I										
11103	L3-L4	Truss 1L	LC	end-I										
11103	L3-L4	Truss 1L	LC	end-I										
11103	L3-L4	Truss 1L	LC	end-I										
11103	L3-L4	Truss 1L	LC	end-I										
11105	L5-L6	Truss 1L	LC	Primar										
11105	L5-L6	Truss 1L	LC	Primar										
11104	L4-L5	Truss 1L	LC	end-J										
11203	U3-U4	Truss 1L	UC	Primar										
11203	U3-U4	Truss 1L	UC	Primar										
11203	U3-U4	Truss 1L	UC	Primar										
11203	U3-U4	Truss 1L	UC	Primar										
11204	U4-U5	Truss 1L	UC	Primar										
11204	U4-U5	Truss 1L	UC	Primar										
11204	U4-U5	Truss 1L	UC	Primar										
11204	U4-U5	Truss 1L	UC	Primar										
21101	L1-L2	Truss 1R	LC	end-I										
21101	L1-L2	Truss 1R	LC	end-I										
21105	L5-L6	Truss 1R	LC	Primar										
21105	L5-L6	Truss 1R	LC	Primar										
21105	L5-L6	Truss 1R	LC	Primar										
21201	U1-U2	Truss 1R	UC	Primar										
21201	U1-U2	Truss 1R	UC	Primar										
21206	U6-U7	Truss 1R	UC	Primar										
21206	U6-U7	Truss 1R	UC	Primar										

**Member Section Loss Datasheet**

Member Number: 12101  
 Section: end-I  
 At Opening?: Y

Data Row: 706  
 Member Name: L1-L2  
 Location: Truss 2L  
 Type: LC  
 Tapered Section?: N

Section No.: 12101b  
 Data Row: 905  
 Section Type: Box

$b_f = 13.00$  in  
 $t_f = 1.00$  in  
 $b_w = 13.00$  in  
 $t_w = 1.00$  in  
 $h_w = 19.38$  in  
 $t_w = 0.88$  in  
 $c = 14.00$  in

Opening Widths  
 Top Flange: - in  
 Bottom Flange: 8.00 in  
 Web: - in

Add Loss Record:

Component =   
 Face =   
 At Opening? =   
 Reference Point =   
 Offset =   
 t =   
 b =

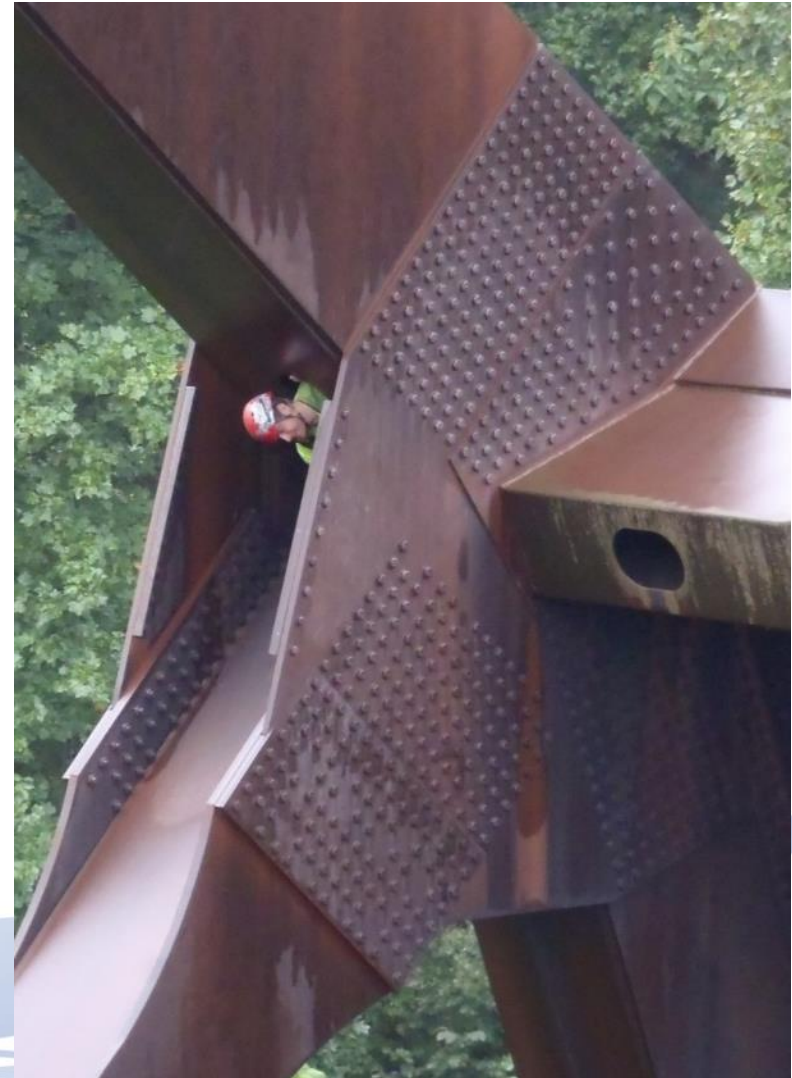
Recorded Section Loss

Data Row	Comp.	Face	At Opening?	Reference Point	Offset (in)	t (in)	b (in)
177	Bot Fl	Inside	Y	Left	0.50	0.19	2.00
178	Bot Fl	Inside	Y	Rt.	0.50	0.19	2.00
179	L. Web	Inside	Y	Bot.	0.00	0.19	4.00
180	R. Web	Inside	Y	Bot.	0.00	0.19	4.00



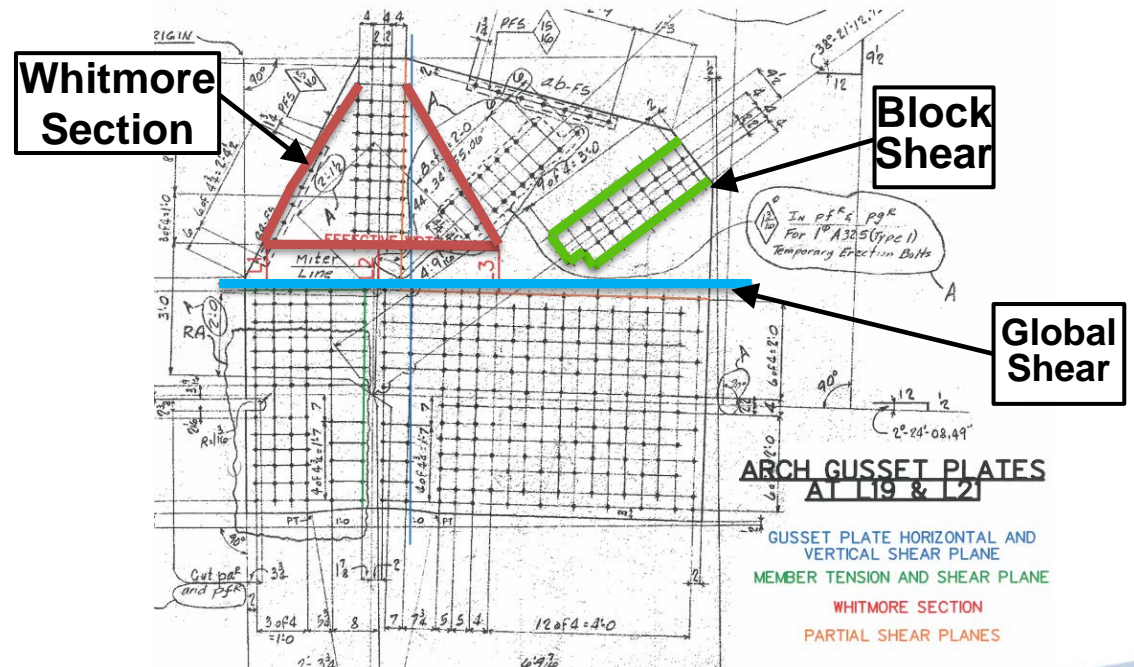
# Gusset Plate Rating

- A total of 871 gusset plates were rated.
- Many of the connections are very large and very complex.
- A standardized data collection and rating system was used for efficiency and consistency.



# Efficient and Consistent Data Collection

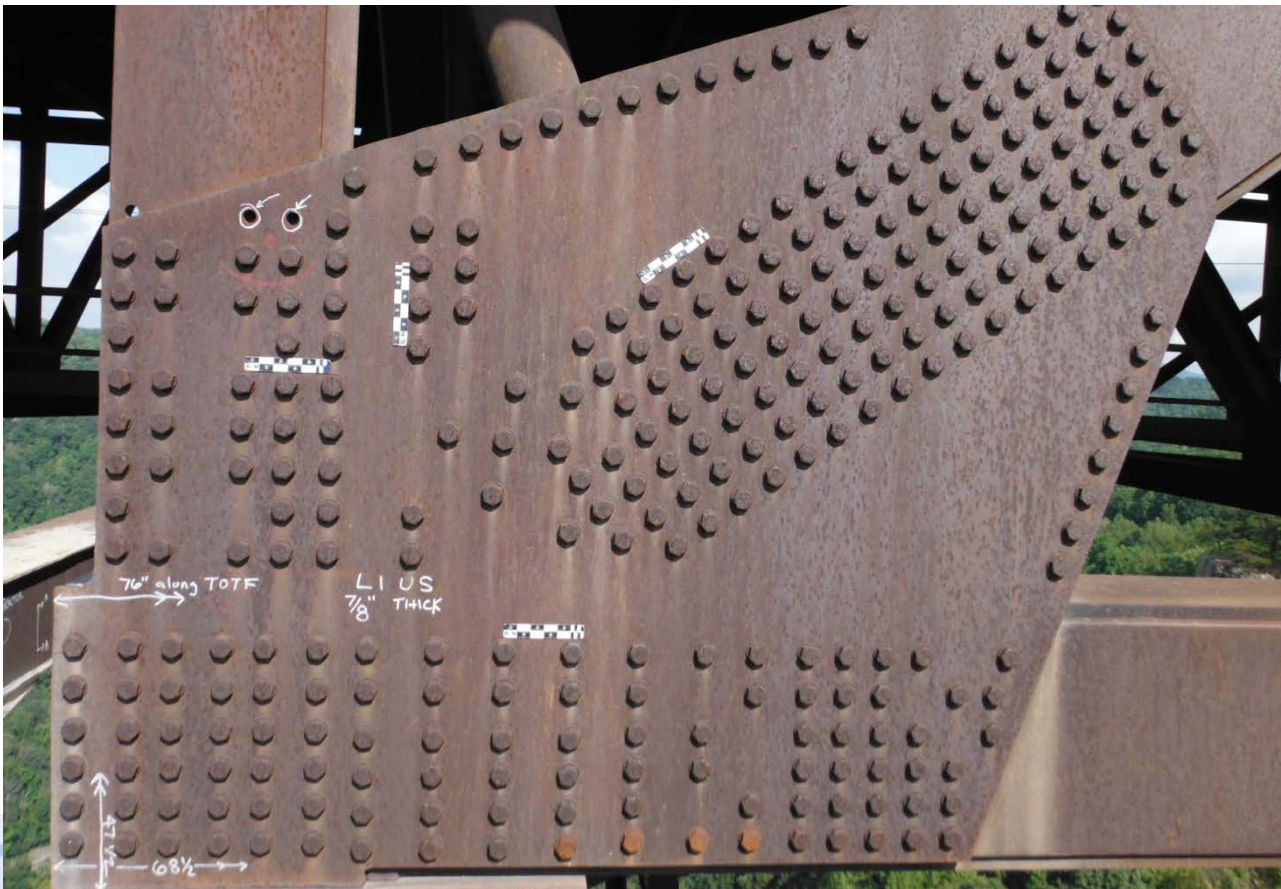
- Shop drawings were imported into Cadd and scaled.
- Critical dimensions (Whitmore section, block shear, global shear planes) were added and measured within the drawing.





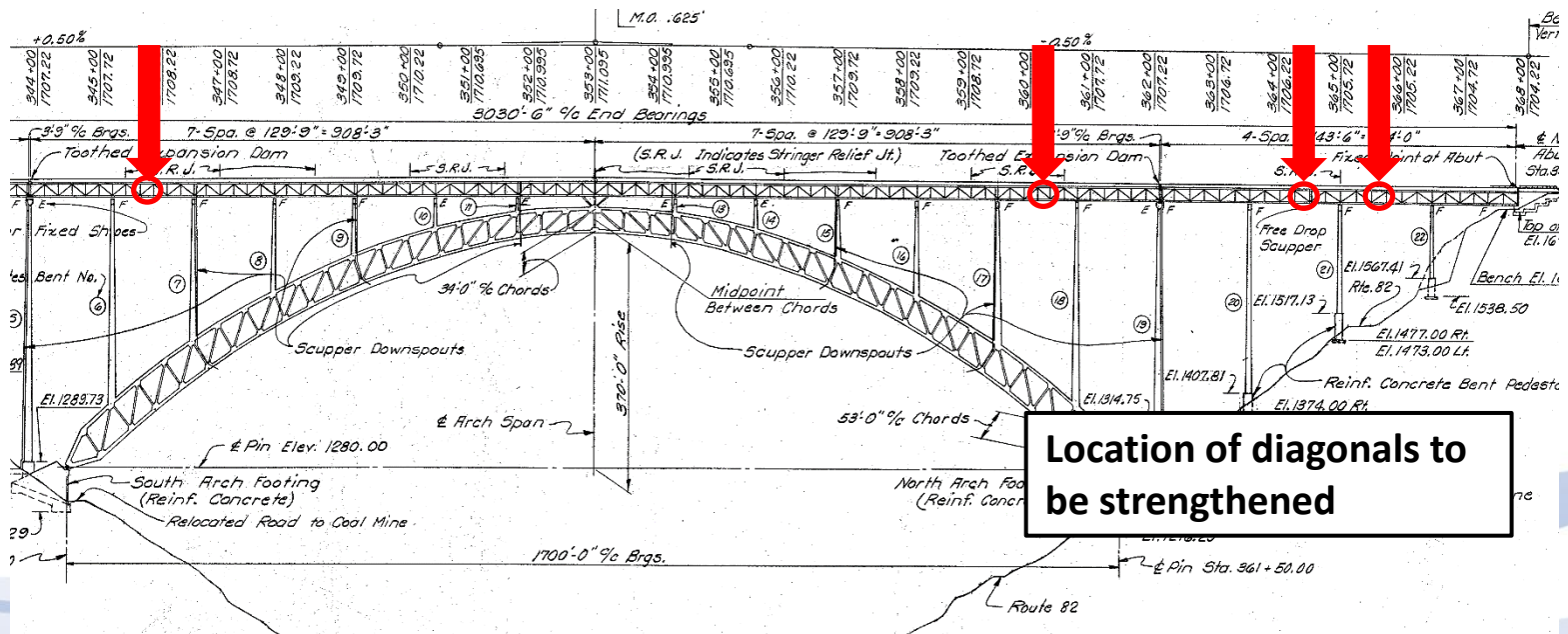
# Gusset Plate Rating

Field data collection was used where plan data was incomplete.



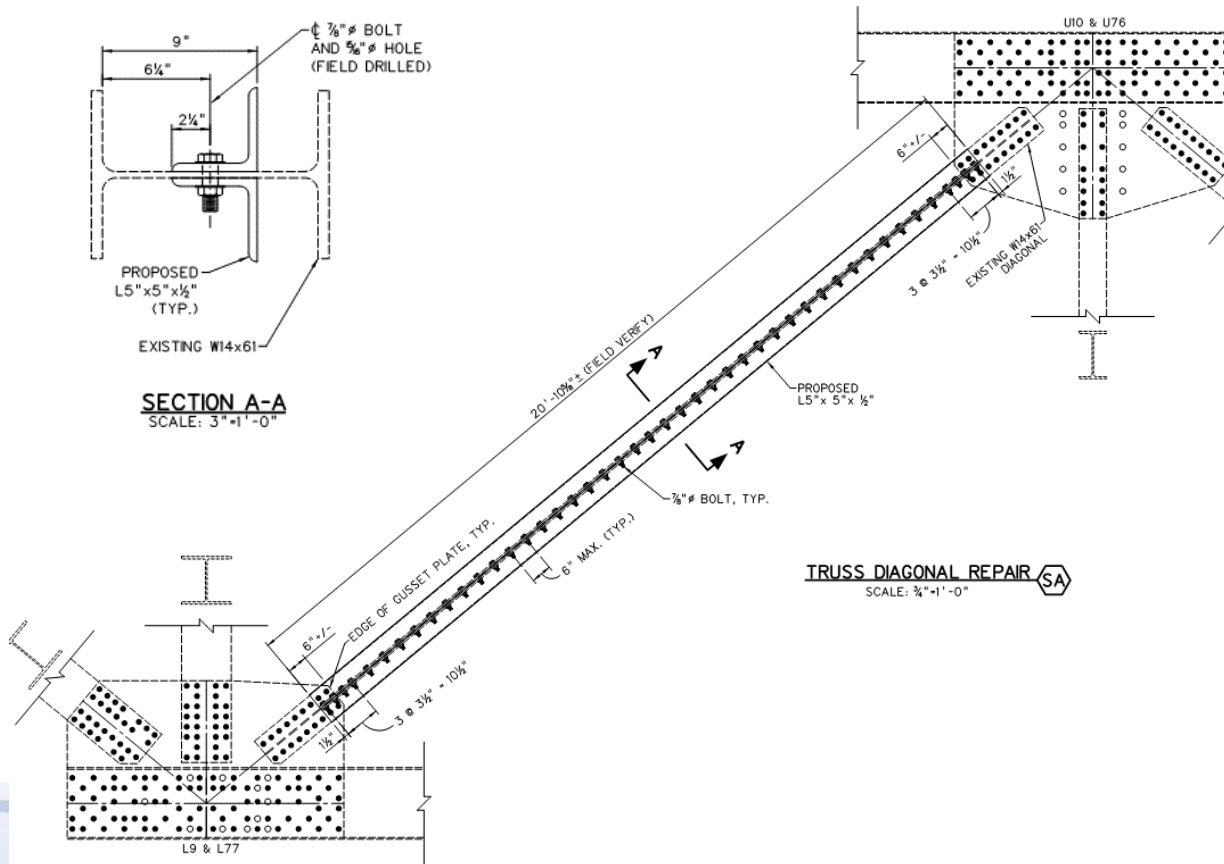
# Rating Results For Existing Bridge

- 8 deck truss diagonals need strengthened
- All rated members have HS20 operating ratings > 1.0
- All gusset plates and connections have HS20 inventory and operating ratings > 1.0



# Member Strengthening

- Weak axis bending strength will be increased by addition of bolted angles.





# 2015 Bridge Rehabilitation

## *Scope of Work*

- Clearing and Grubbing
- Concrete Sealing and Patching
- Clean and Paint Selected Areas
- Replace Deteriorated Bolts
- Solve Debris Issue in the Arch Members
- Replace Abutment and Stringer Relief Joints
- ➔ Strengthen Deck Truss Diagonals
- Retrofit and Reset Bearings at Bents 19 and 5
- Miscellaneous Repairs and Maintenance, etc.



# Clearing Trees and Vines Below the Bridge

- Trees > 5" Dia. Within 50' of bridge
- Trees growing into Bridge
- Trees falling onto Bridge
- Trees preventing safe inspection
- Vines on Bridge
- Leave it lay, out of the way
- Leave paths with room to work/inspect



**North Abutment**





# “Just-in-Time” Substructure Inspection

- 100% Sounding
- Baskets/Rappelling
- Mark w/Paint
- Photograph/Document/Quantify

Green = Crack Seal  
Orange = Patching





# Finished Product

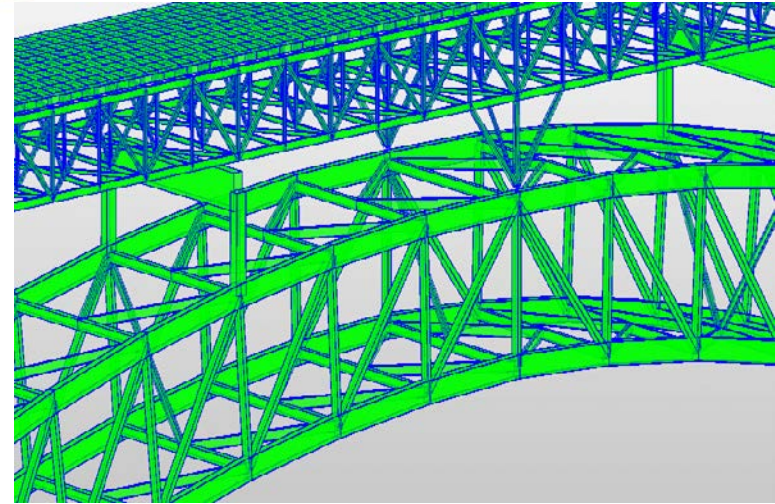


- Cracks – Two-part hybrid urethane mender
- Patches - Non-shrink, High Early Strength
- Protective Epoxy Coating



# Primary Issue = Section and Nut Loss

- ❖ Deck Truss
  - Sealed in 2012
  - Punch list locations
- ❖ Bents and Arch
- ❖ Cleaning and Sealing
- ❖ Follow-up with Magnets
- ❖ Not Controlling the Load Rating



BENT BASE

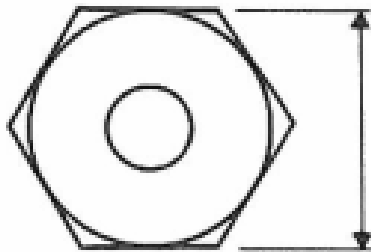


ARCH DIAGONAL

# “Lesson Learned” from Previous Rehabs

## BOLT REPLACEMENT GUIDE

1. THE FOLLOWING GUIDES SHALL BE USED TO DETERMINE THE NEED FOR BOLT REPLACEMENT AT LOCATIONS NOT MARKED IN THE FIELD AND/OR NOT SHOWN IN THE PLANS:



"N" = FLAT TO FLAT DIMENSION OF NUT

DECK TRUSS/BENT BOLTS: ORIGINAL BOLT DIAMETER = 7/8"

"N"

1 3/8" = 0% LOSS (AS MEASURED IN THE FIELD)

1 1/8" = 50% LOSS = REPLACE

ARCH BOLTS: ORIGINAL BOLT DIAMETER = 1 1/8"

"N"

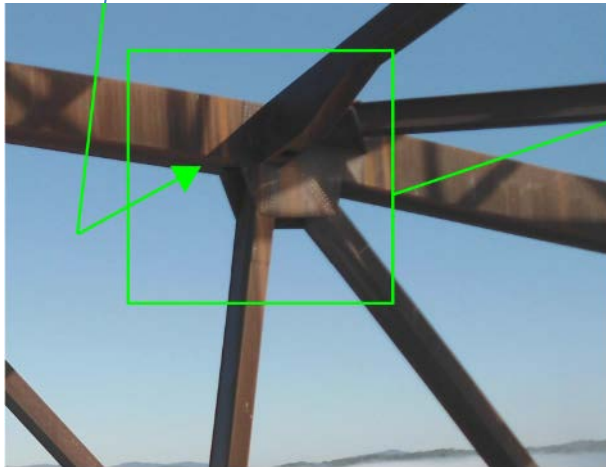
1 13/16" = 0% LOSS

1 1/2" = 50% LOSS = REPLACE



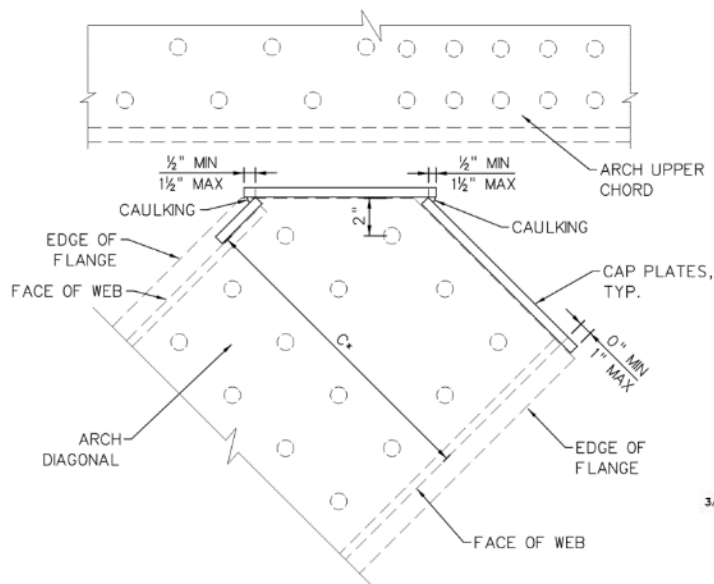
# Solving Debris Issues in Arch Members

- **Problem:** Water Flowing Under Arch Rib
- **Solution:** Divert Water with Magnetic Drip Bar



# Solving Debris Issues in Arch Members

- **Problem:** Water/Birds Entering Top of Arch Members
- **Solution:** Install HDPE Cap Plates

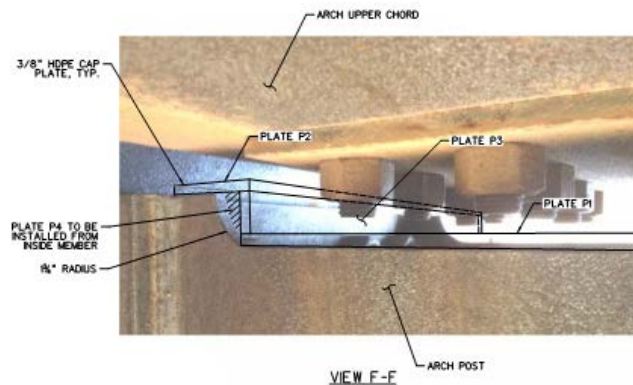


C\* = 1'-4<sup>7</sup>/<sub>8</sub>" FOR DIAGONALS EXCEPT AT THE FOLLOWING LOCATIONS:  
 2'-1<sup>1</sup>/<sub>8</sub>" @ U2 & U38  
 1'-9<sup>1</sup>/<sub>8</sub>" @ U5, UB, U35, & U32  
 1'-5<sup>1</sup>/<sub>8</sub>" @ U11, U29, & U20  
 1'-5" @ U14, U17, U26, & U23

**DETAIL C**  
 SCALE: 3"=1'-0"



**TYPICAL VIEW**  
 LOOKING DOWN AND NORTH



**VIEW F-F**

# Solving Debris Issues in Arch Members

- Installed HDPE Cap Plates





# Solving Debris Issues in Arch Members

- **Problem:** Members Not Draining, Weep Holes too High

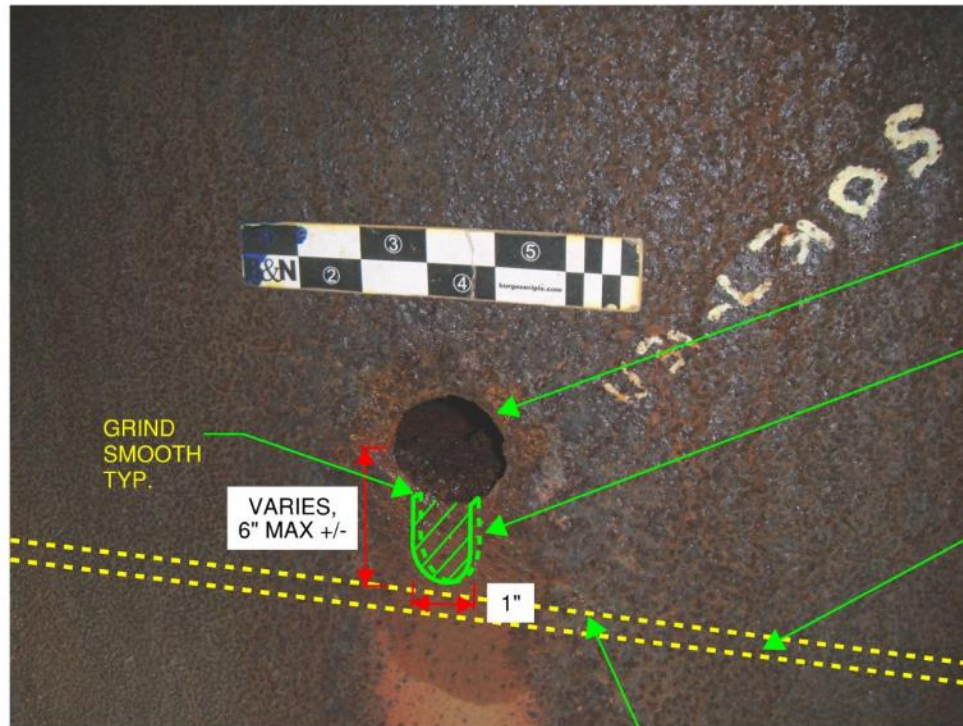


TYPICAL UPPER ARCH WEEPHOLE



# Solving Debris Issues in Arch Members

- **Problem:** Members Not Draining, Weep Holes too High
- **Solution:** Extend Weep Holes



GRIND SMOOTH TYP.

VARIABLE, 6" MAX +/-

1"

DETAIL A

1 1/2" DIA. WEEP HOLE, TYP.

(WH)

1/2" END DIAPHRAGM PLATE, TYP.

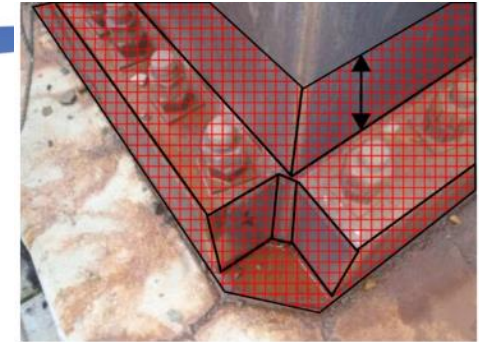
DO NOT CUT INTO TOE OF END DIAPHRAGM WELD





# “Danielson! Show me Wax On... Wax Off...”

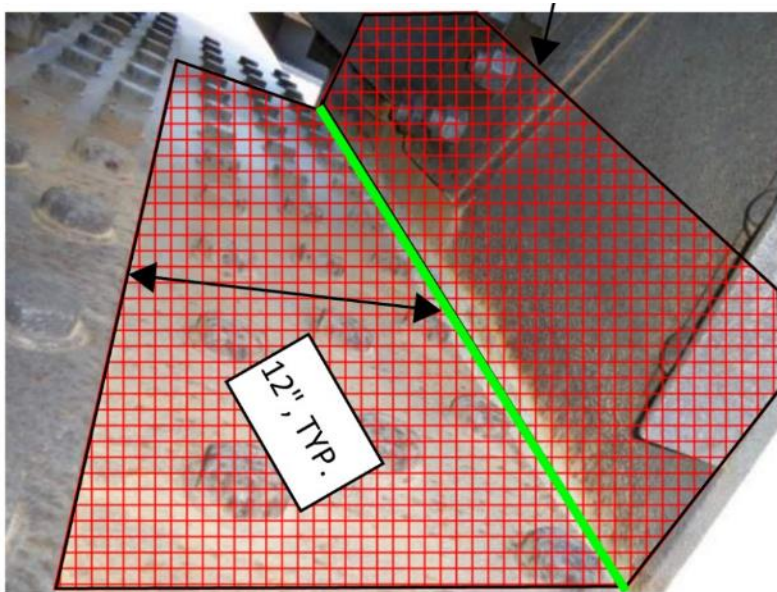
- Pressure Wash or Power Tool
- Penetrant Sealer – let it penetrate 15 mins.
- Wax Coating – sets up like a candle wax
  - Two on WVDOH Approved Products list



EXTERIOR VIEW

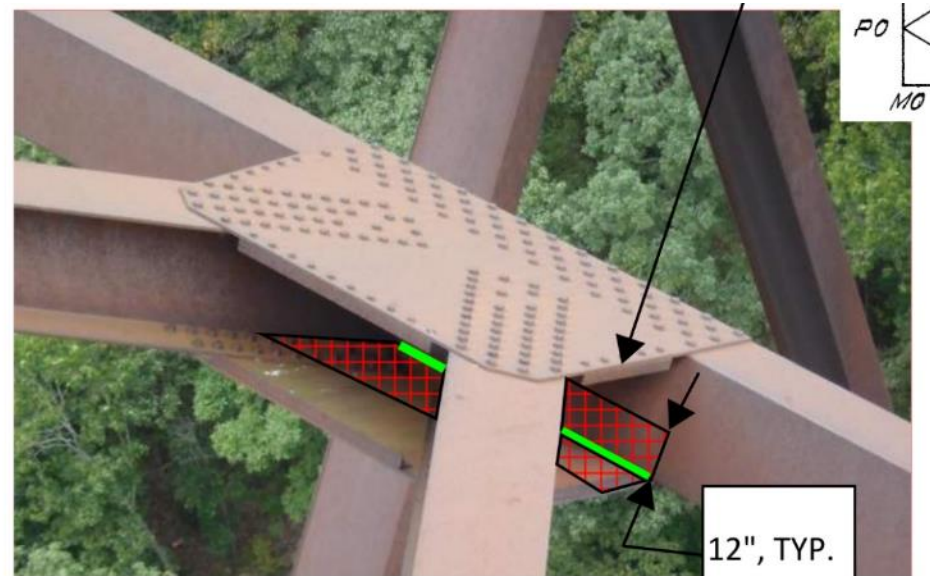
**MAX-WAX**

**CHEMICAR<sup>USA</sup> TEKTON 34**

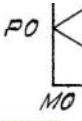


TYPICAL W & P CONNECTION WITH VERTICAL GUSSET

**BURGESS & NIPLÉ**

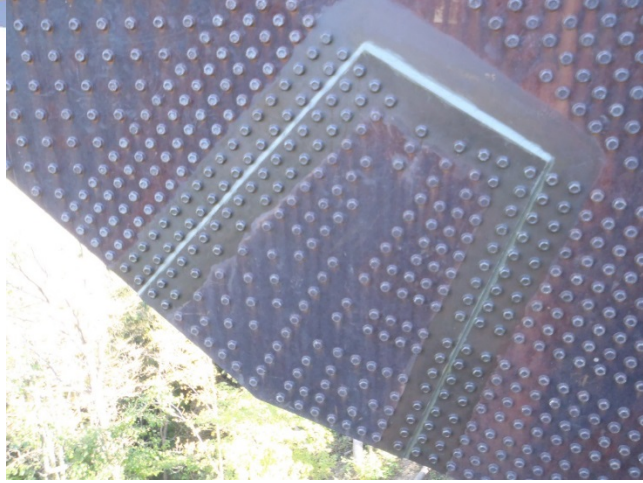


TYPICAL W CONNECTION





# Looking GOOD!



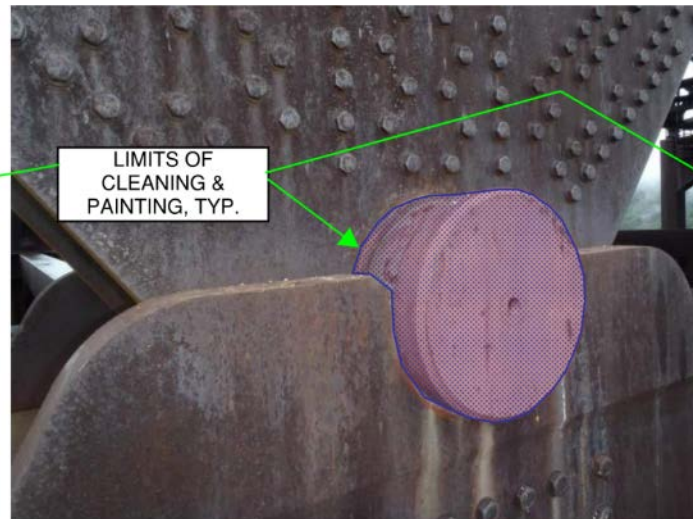
# Looking GOOD!





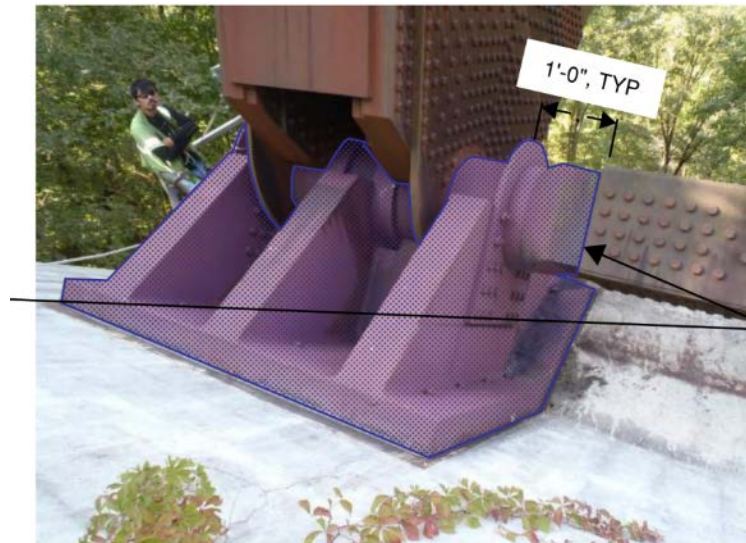
# A few places are not weathering steel

- Power Tool Clean
- Epoxy Mastic Prime
- Urethane Top



TYPICAL ARCH PIN AT U20 OF ARCH  
(OUTBOARD VIEW)

3 IN KEY



TYPICAL ARCH GRILLAGE AND PIN AT M0 & M40 US/DS  
(INBOARD VIEW)

CLEANING SHALL BE PER SSPC-SP 15. EPOXY MASTIC PRIME COAT SHALL BE 5 MILS MINIMUM DRY FILM THICKNESS. URETHANE TOP CHOAT SHALL BE 3 MILS MINIMUM DRY FILM THICKNESS.

LIMITS OF  
CLEANING &  
PAINTING,  
TYP.



# Contractor's Access – Not What We Expected



- Spiders
- Anchored to:
  - Catwalk or
  - Rolling Frames on Deck
- Challenging for Inspectors
- B&N Onsite



# Leaking Joints causing Pack Rust at Bearings



# Expanding Polyurethane Foam Joints

PRIOR TO INSTALLING THE PROPOSED ABUTMENT JOINT, THE TOP 1" OF VERTICAL LEG AND ALL OF THE 8" HORIZONTAL LEG OF EXISTING ANGLE IS TO BE SAND BLASTED TO A NEAR WHITE FINISH (SSPC-SP 10/NACE NO. 2) AND METALIZED PER SECTION 689 OF THE STANDARD SPECS. THE SEALER AND TOP COAT CAN BE ELIMINATED FOR THIS APPLICATION - BOTH SIDES.

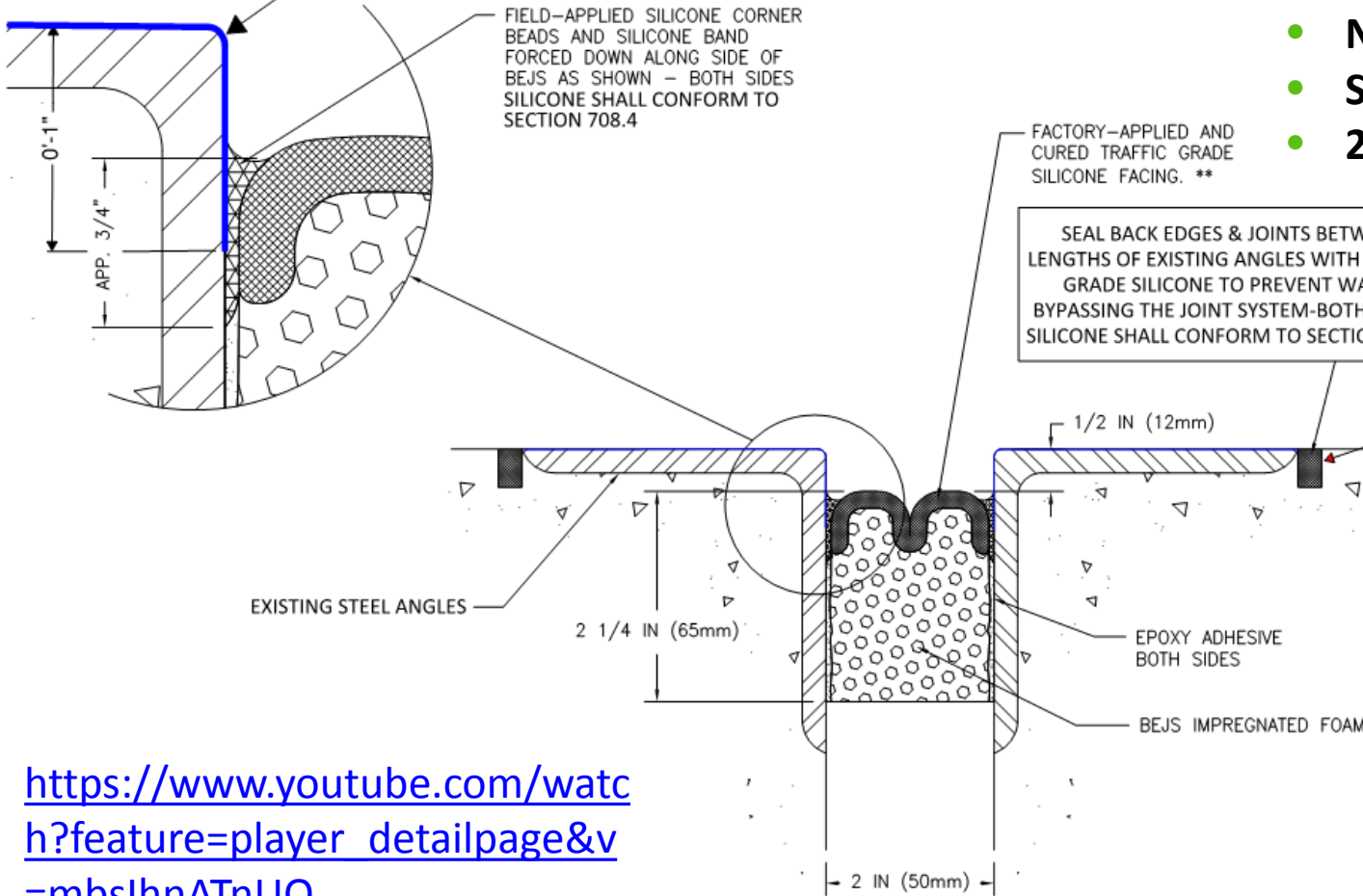
FIELD-APPLIED SILICONE CORNER BEADS AND SILICONE BAND FORCED DOWN ALONG SIDE OF BEJS AS SHOWN - BOTH SIDES SILICONE SHALL CONFORM TO SECTION 708.4

FACTORY-APPLIED AND CURED TRAFFIC GRADE SILICONE FACING. \*\*

SEAL BACK EDGES & JOINTS BETWEEN LENGTHS OF EXISTING ANGLES WITH TRAFFIC GRADE SILICONE TO PREVENT WATER BYPASSING THE JOINT SYSTEM-BOTH SIDES. SILICONE SHALL CONFORM TO SECTION 708.4

## Features

- Silicone Impregnated
- Traffic durable
- Pre-compressed
- Non-invasive anchoring
- Staged installation
- 2 lanes in 2 hours



### NOTES:

THE SEAL JOINTS SHALL BE CONSTRUCTED BY SAWING 1/2" DEEP ALONG FULL LENGTH OF THE STEEL ANGLES.

THE USE OF AN EDGE GUIDE, FENCE, OR JIG IS REQUIRED TO ENSURE THAT THE CUT JOINT IS STRAIGHT, TRUE, AND REMAINS IN CONTACT WITH THE STEEL ANGLE. THE JOINT WIDTH SHALL BE THE WIDTH OF THE SAW BLADE, NOMINAL WIDTH OF 1/4 INCH.

[https://www.youtube.com/watch?feature=player\\_detailpage&v=mbsIhnATnUQ](https://www.youtube.com/watch?feature=player_detailpage&v=mbsIhnATnUQ)

**PROPOSED ABUTMENT JOINT DETAIL (EJ1)**



# Deck Repairs....As Simple as 1, 2, 3.....

1



2



3



4



# Bearing Slide Video

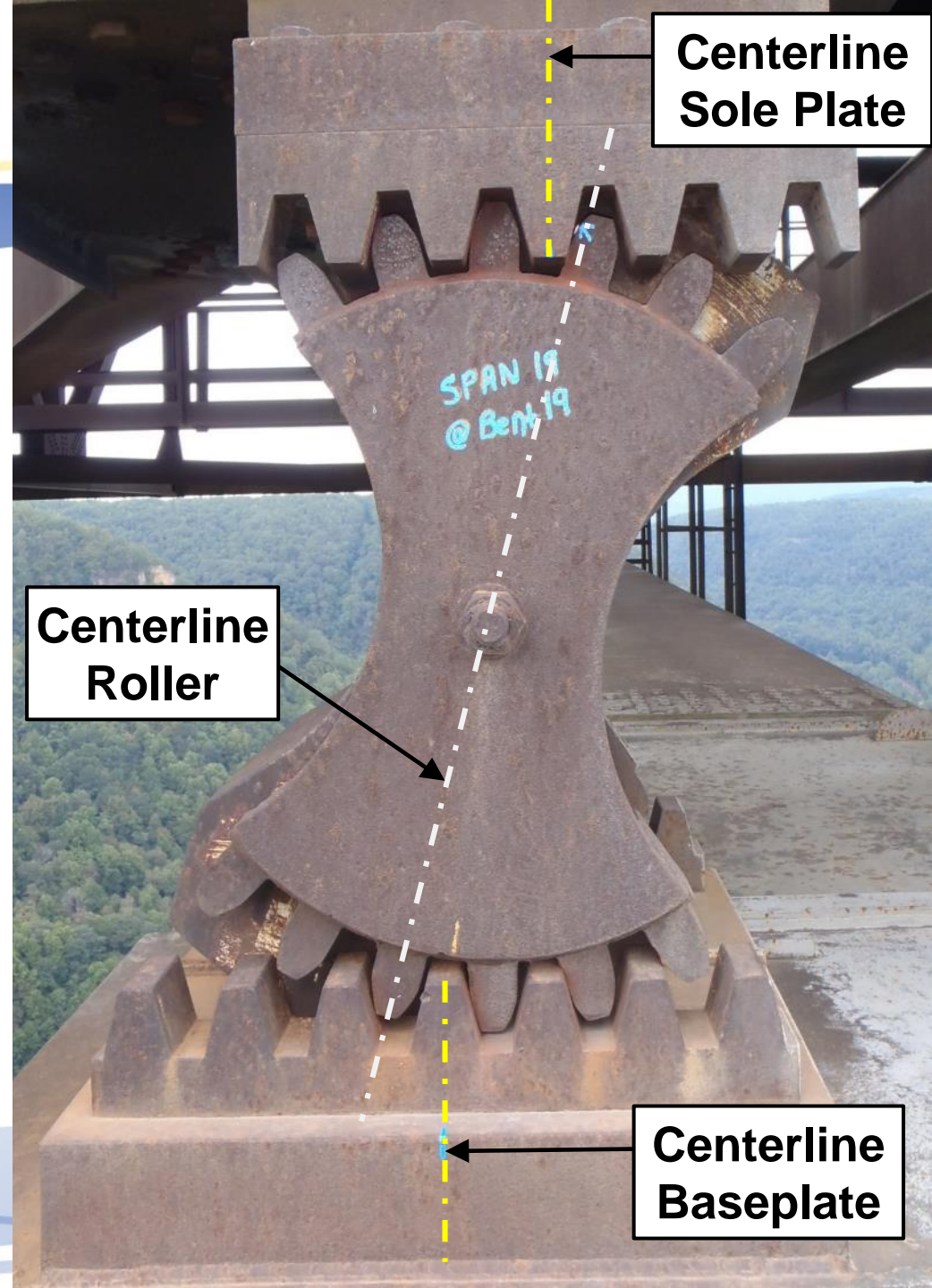


Prepared by Maximus Films Production for the German  
Documentary on the Worlds Most Extreme Bridges



# Bearing Repositioning

- Excessive tilt.
- Plates were misaligned at mean temperatures.
- Roller had slipped.
- Retainer plates should prevent slippage, but they were not functioning properly.





# Bearing Repositioning

- Geared retainer plates are intended to keep bearing from slipping relative to the baseplate and sole plate.
- Retainer plates are connected to the roller only at the center of the roller.
- This connection allows the plates to rotate relative to the roller, making the retainer plates ineffective.



**Retainer Plate Connection**

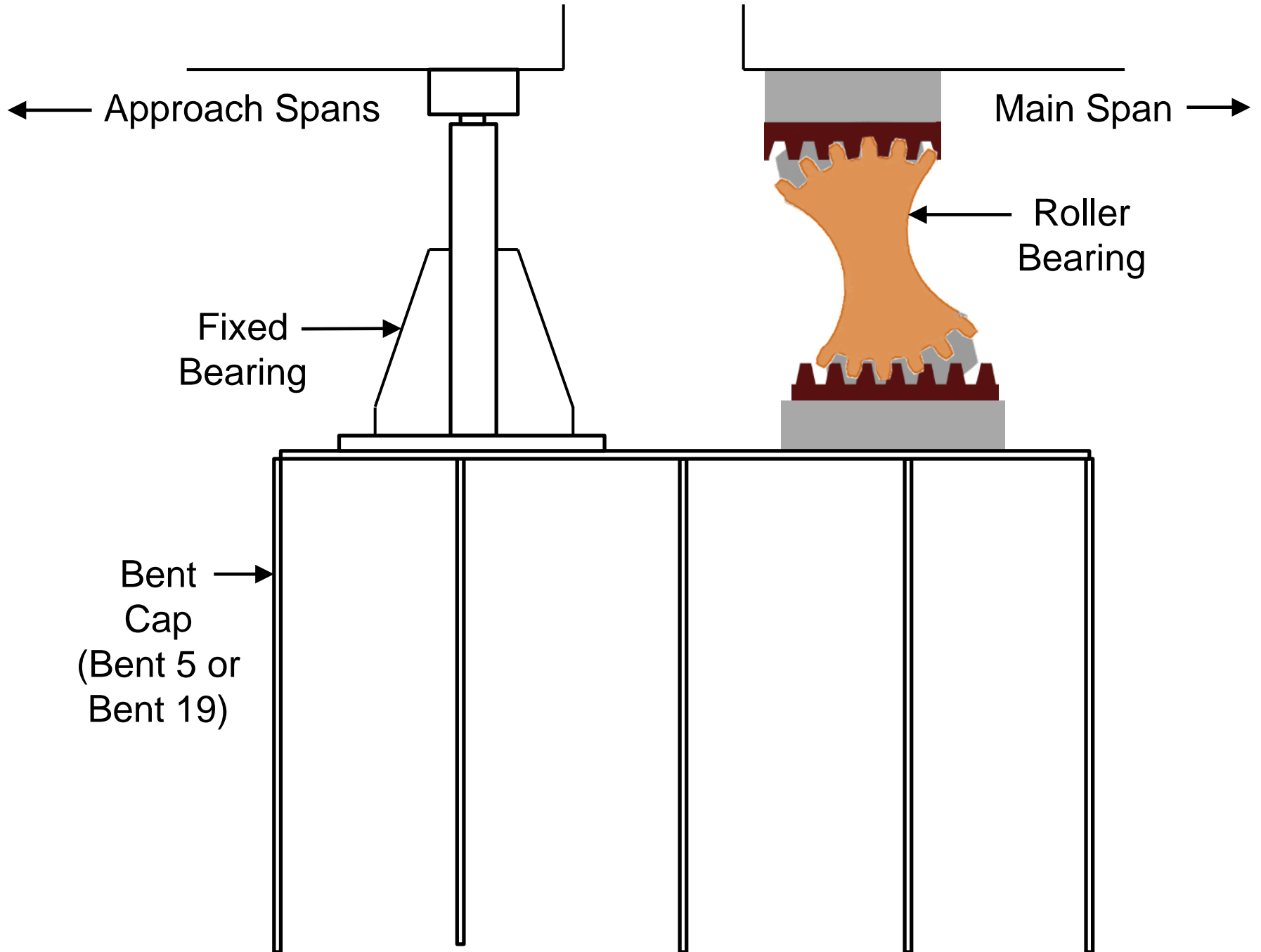
# Vertical Jacking vs. Sliding

- Vertical jacking would be very difficult due to height of the bent and the steel pier cap.

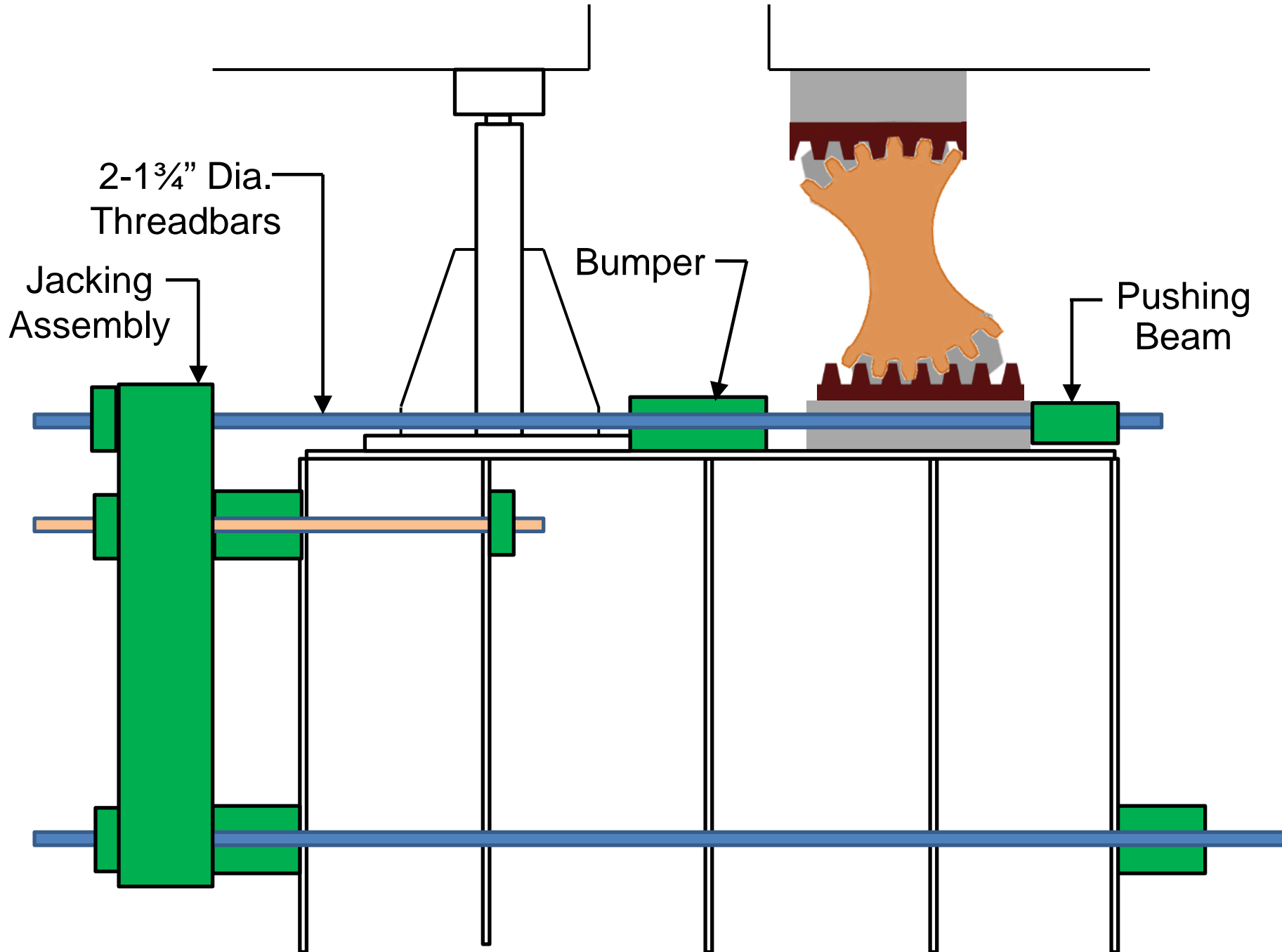




# Schematic layout of Bent 5 / Bent 19 bearings:

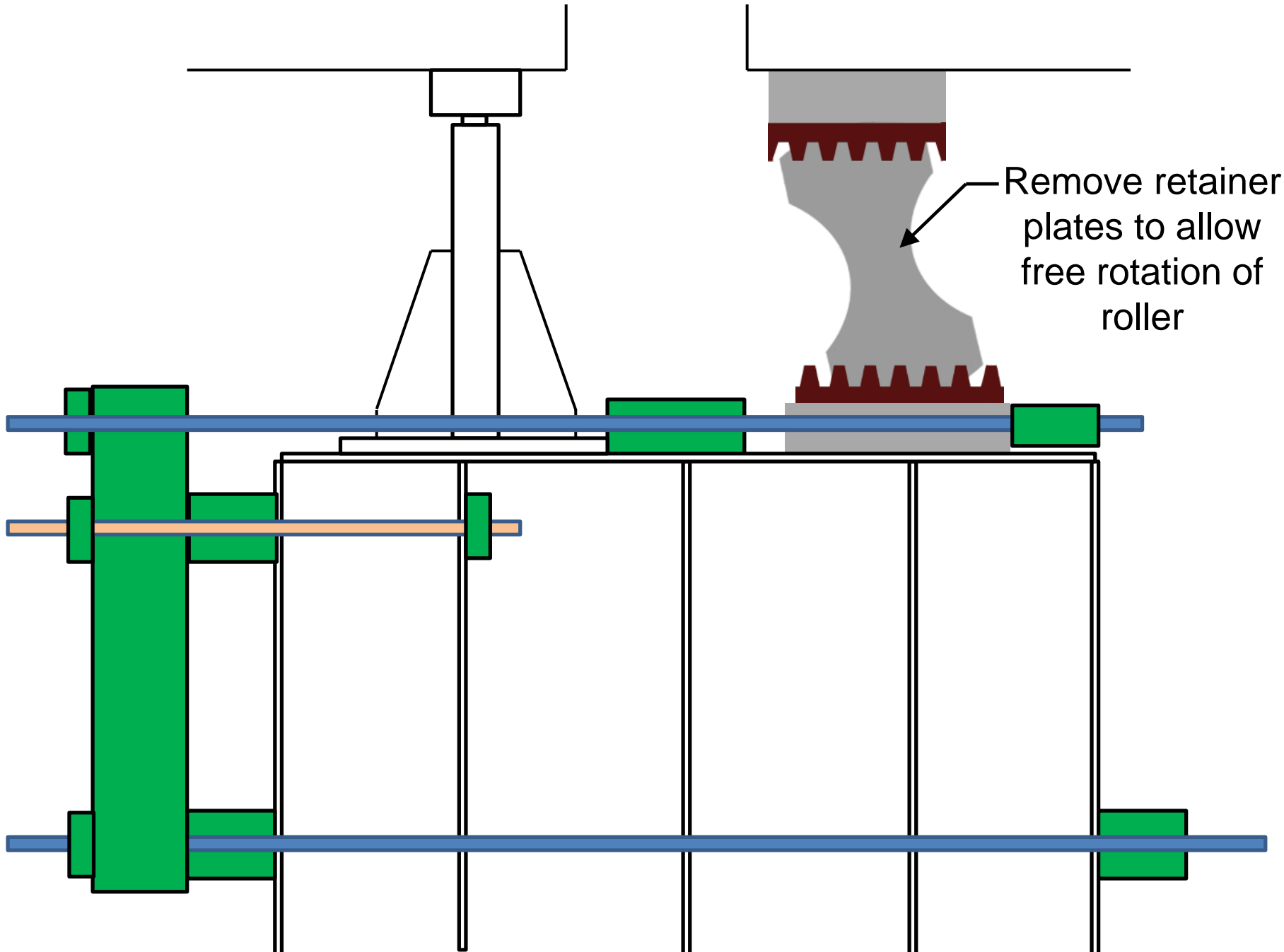


# Step 1: Install Jacking Assembly & Threadbars

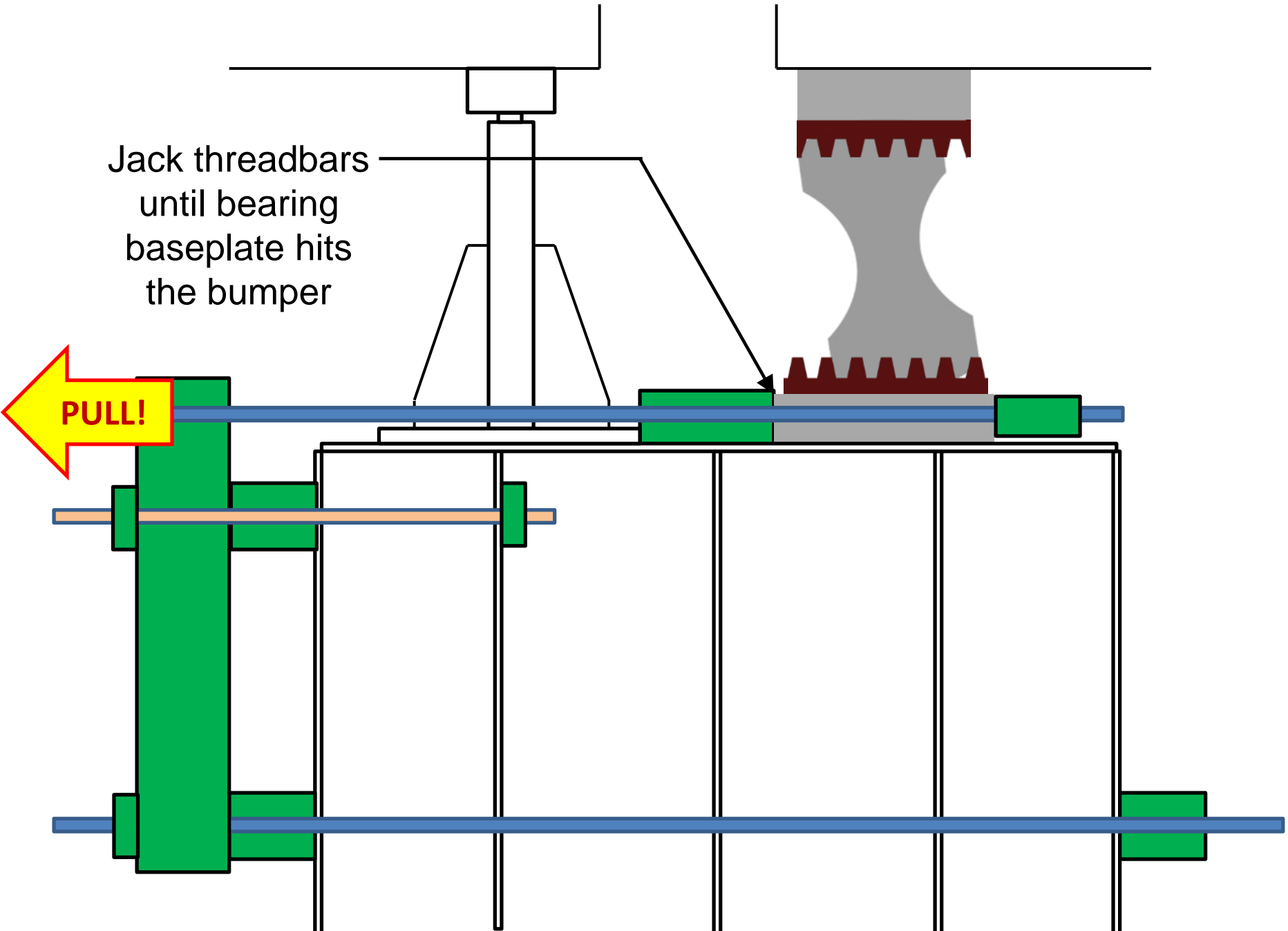




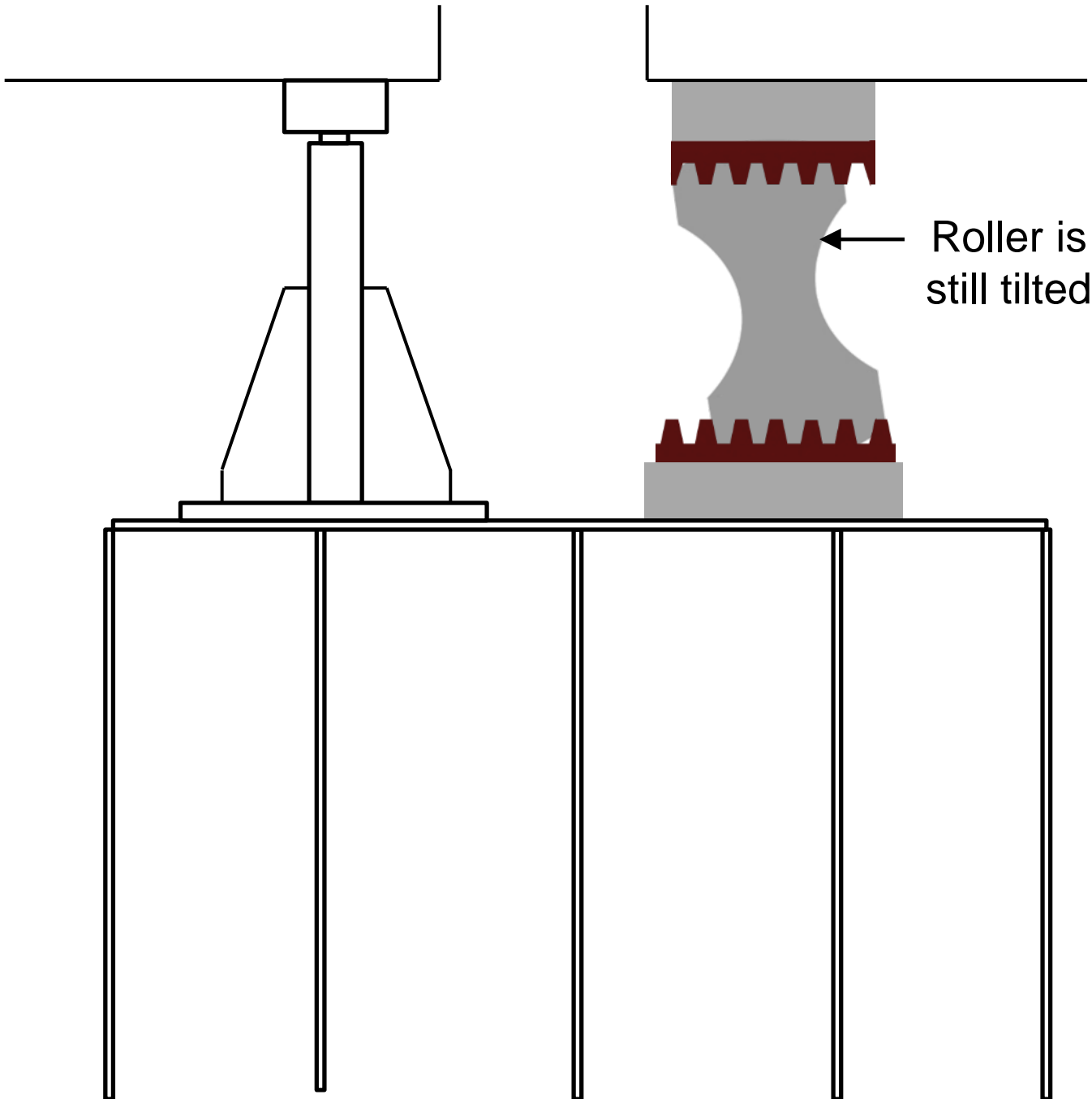
# Step 2: Temporarily remove retainer plates from roller bearing



### Step 3: Jack threadbars, pull bearing base plate to final position

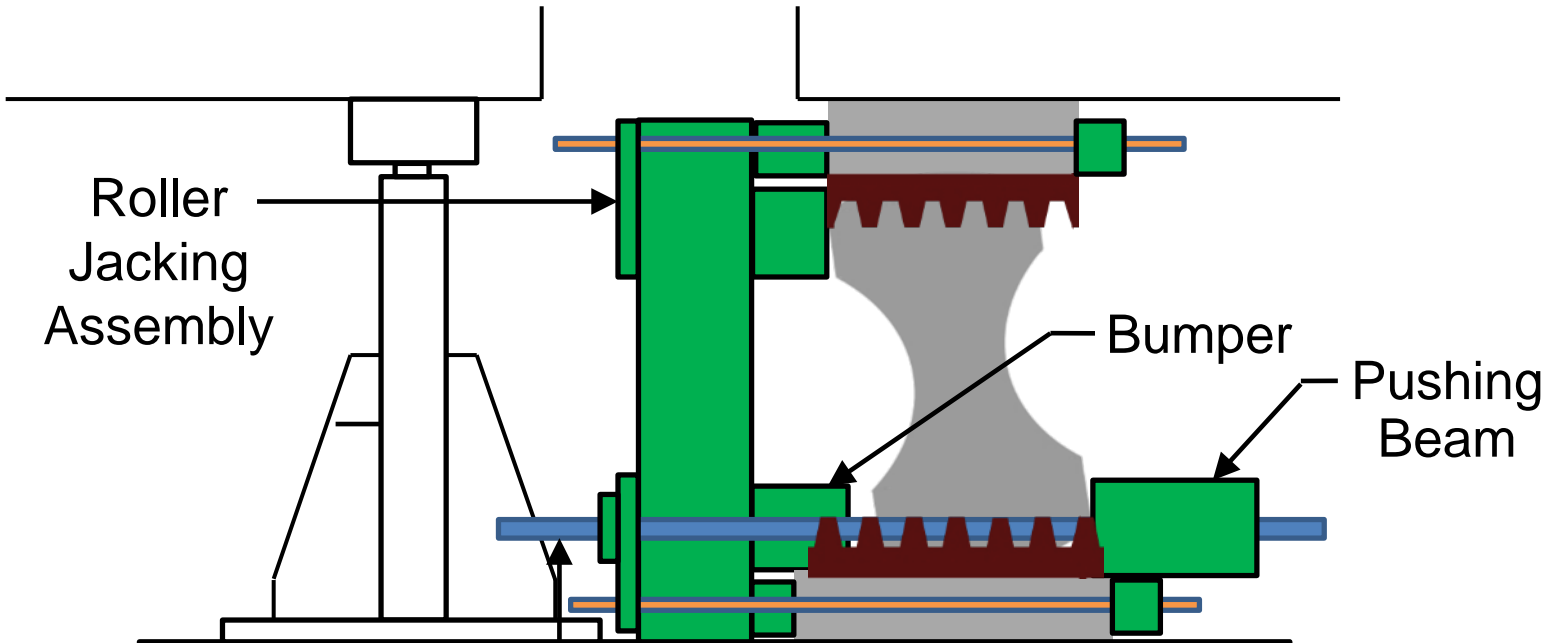


**Step 4: Remove jacking assembly**





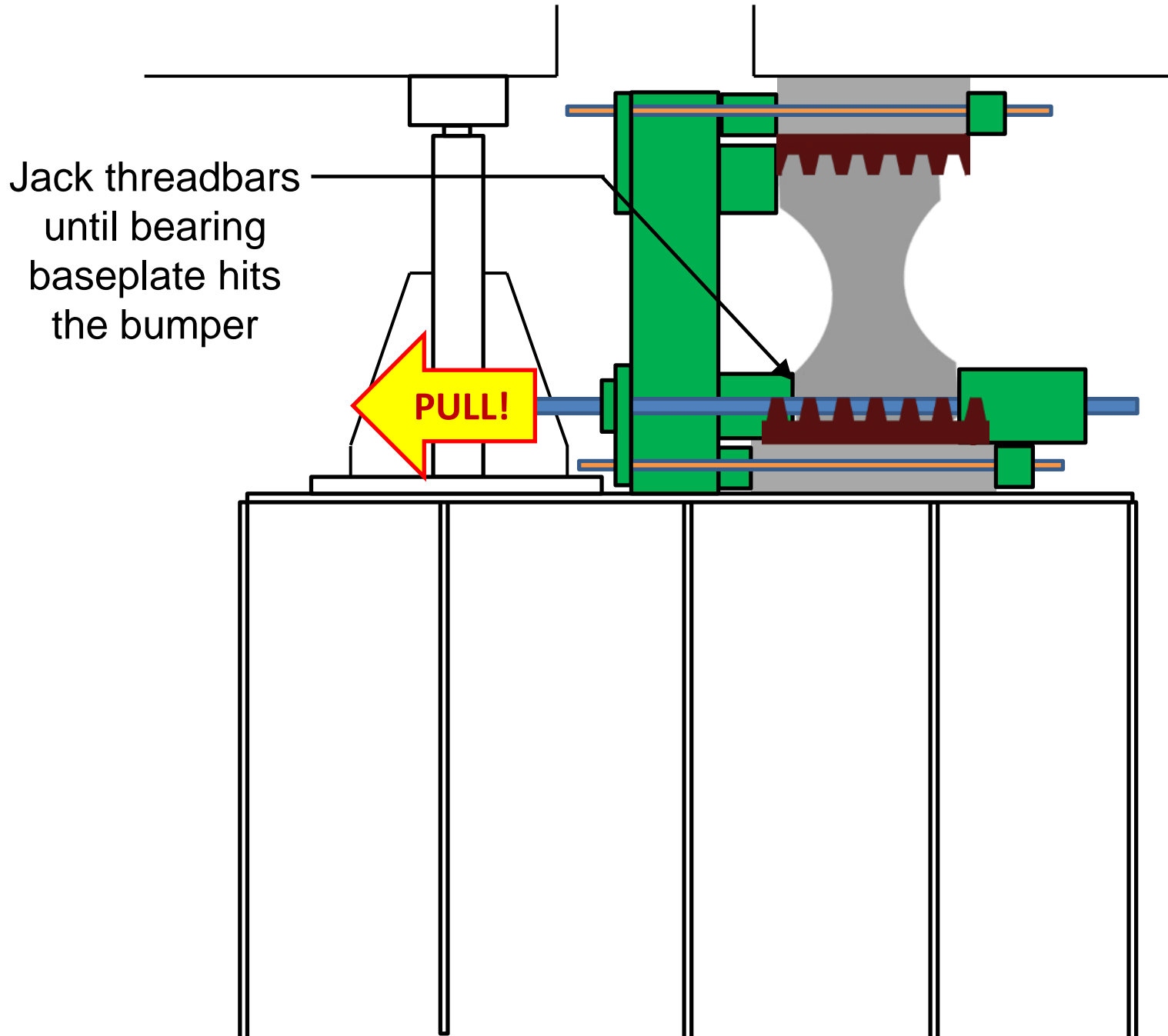
# Step 5: Install roller jacking assembly and threadbars



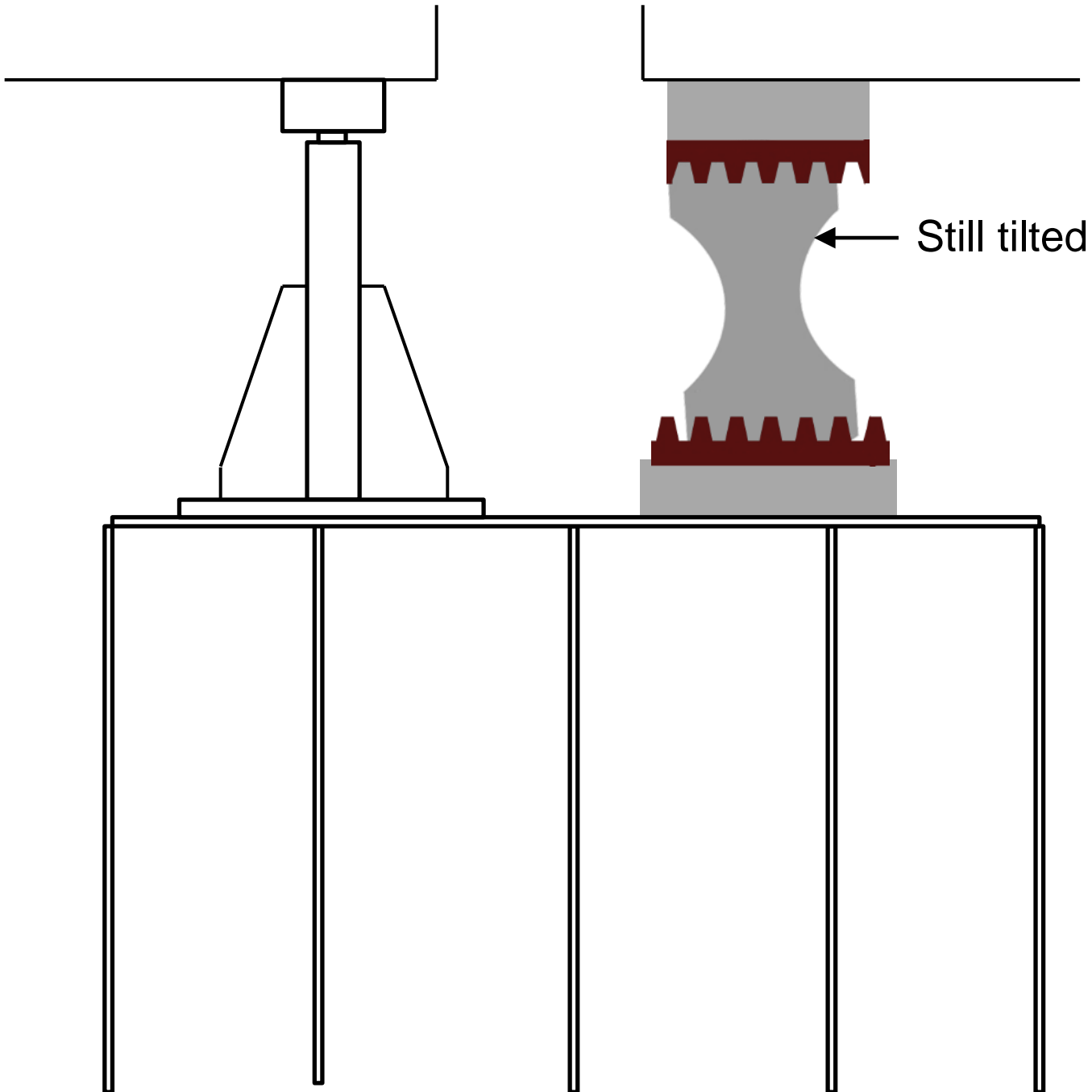
2-1<sup>3</sup>/<sub>4</sub>" Dia.  
Threadbars



# Step 6: Jack threadbars and pull roller base to final position

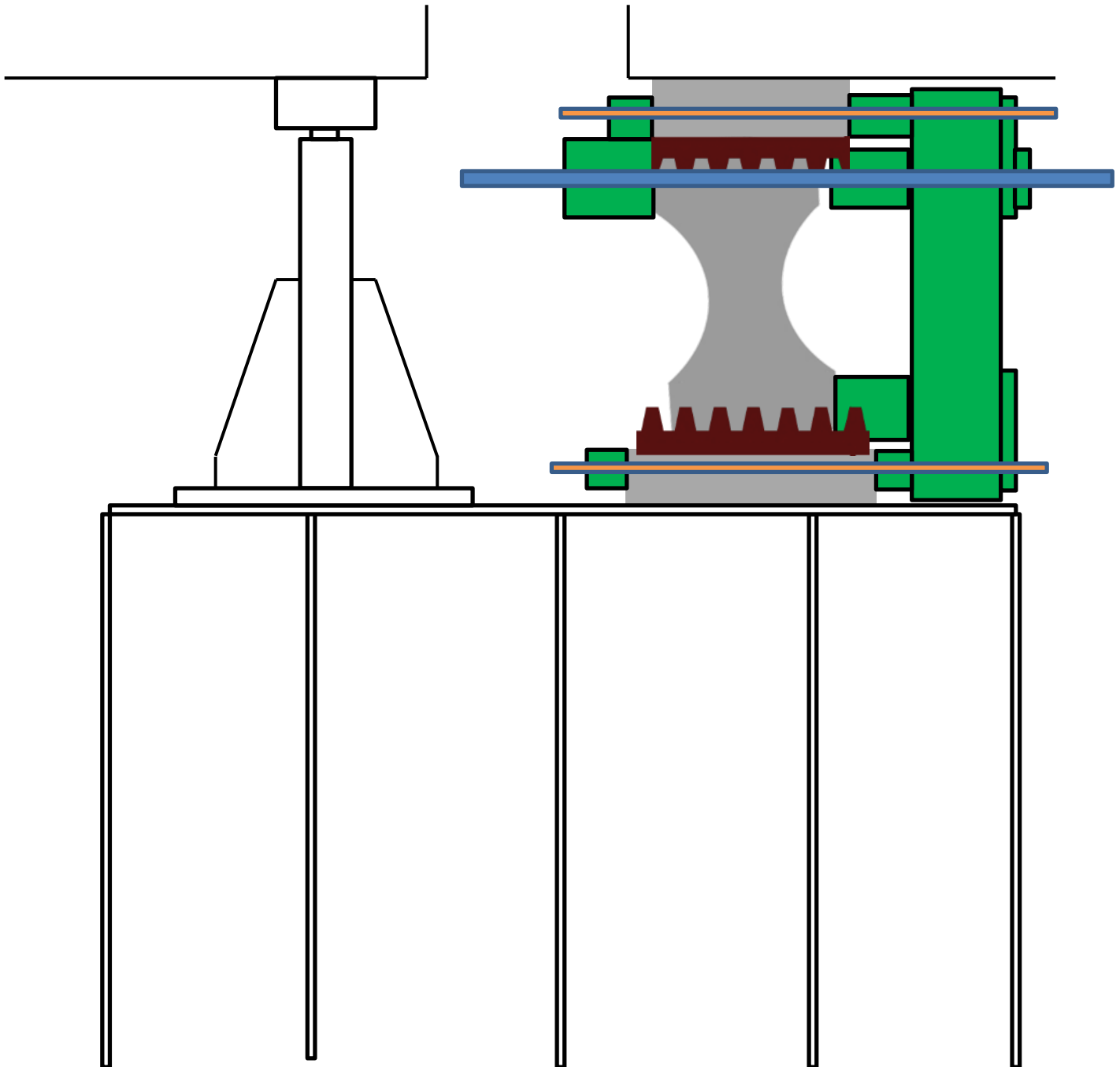


**Step 7: Remove roller jacking assembly**

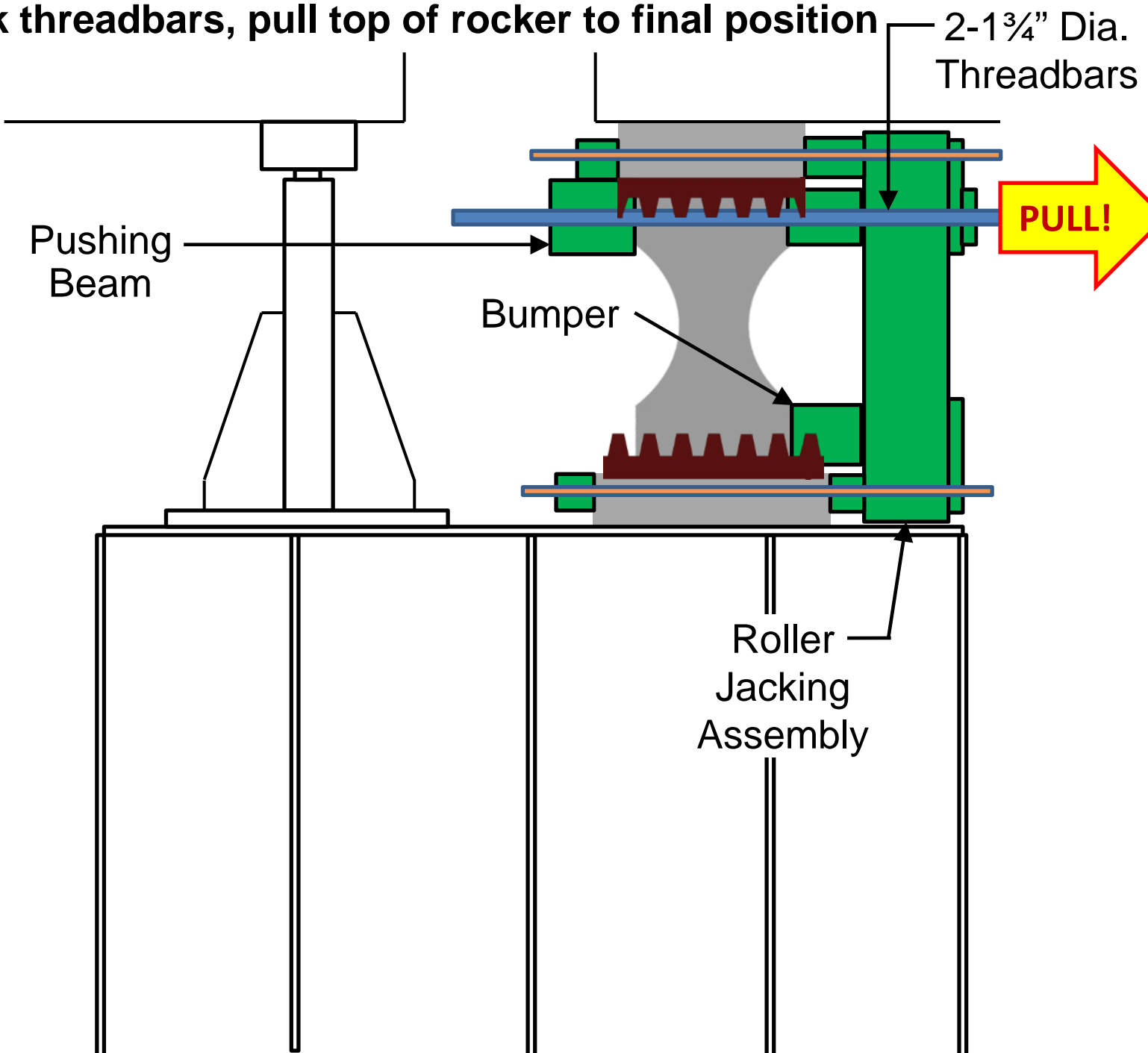




**Step 8: Re-install roller jacking assembly**



**Step 9: Jack threadbars, pull top of rocker to final position**



2-1<sup>3</sup>/<sub>4</sub>" Dia.  
Threadbars

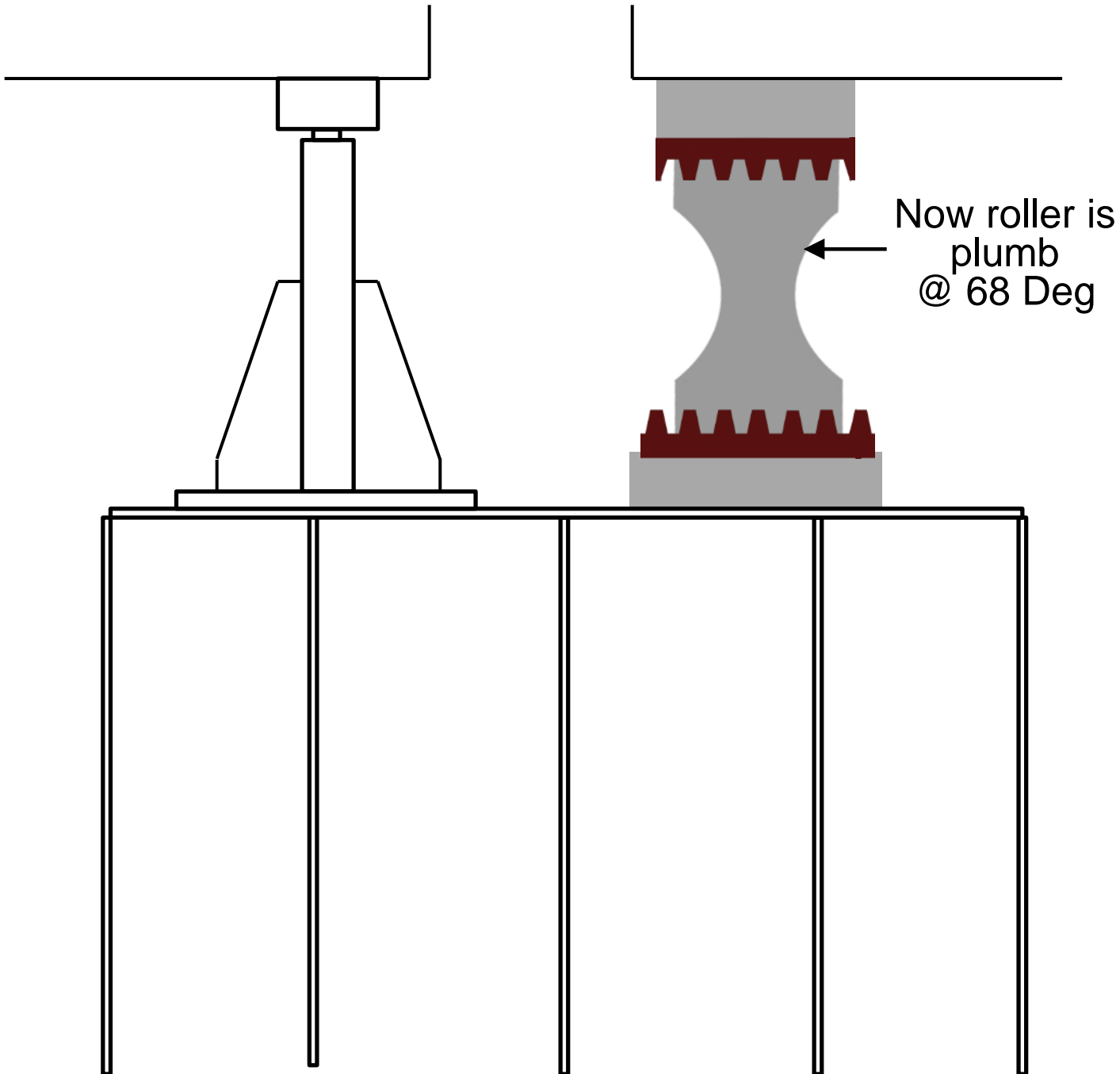
**PULL!**

Pushing  
Beam

Bumper

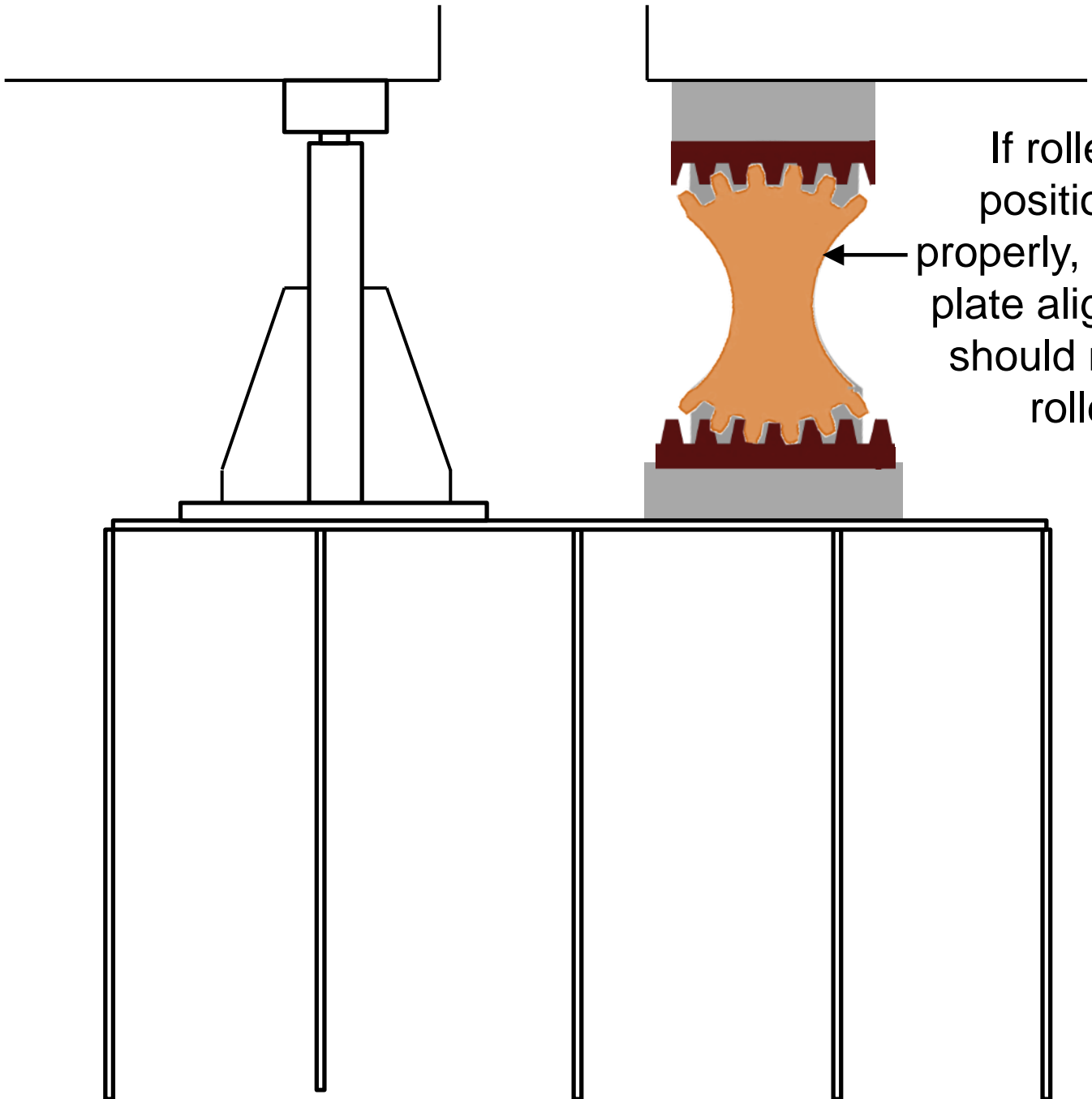
Roller  
Jacking  
Assembly

**Step 10: Remove roller jacking assembly**

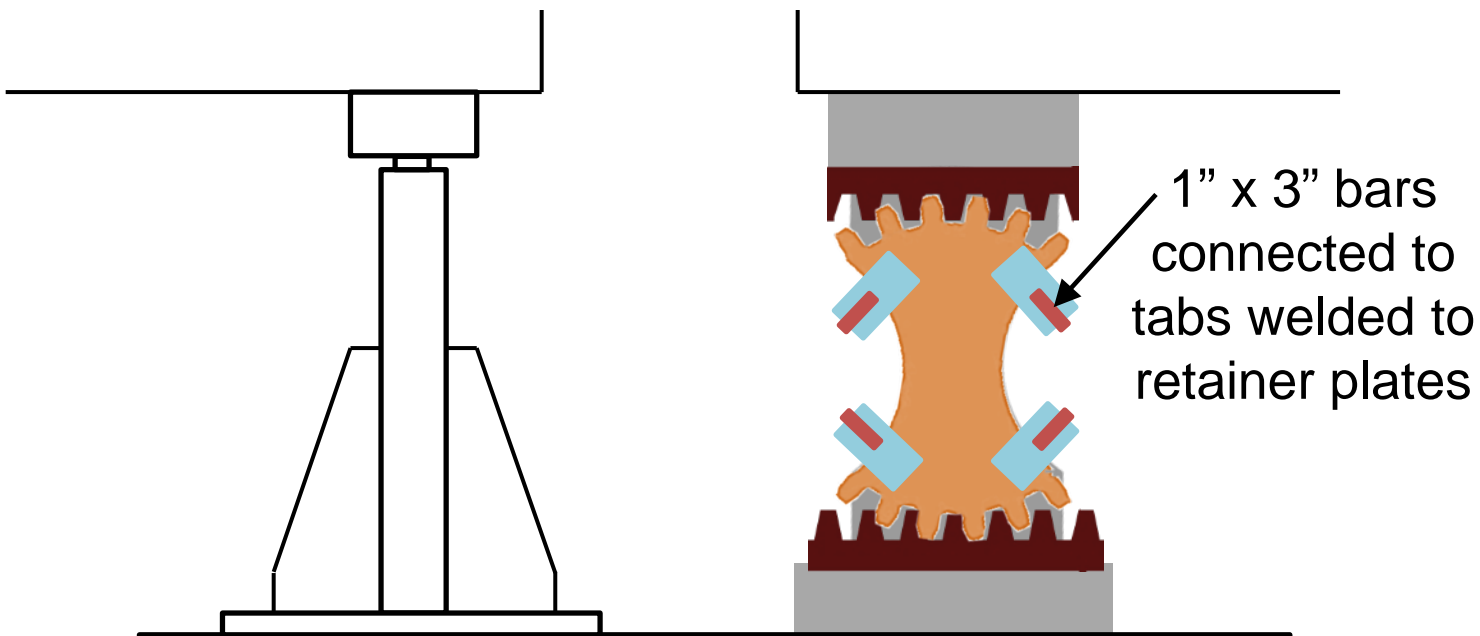




# Step 11: Reinstall retainer plates



# Step 12: Install retaining bars to lock retainer plates and roller together



# QA/QC Inspection - Punch List Reports



**Photo 16**

Typical area where sealer needs touched up at outside fillet weld channels and at bottom of vertical gusset plate inside spandrel bent bottom cell.



At bases of land bents, touch up the paint on the concrete surface around the steel bases as needed.



# Today's Take Away's

- Significant, Unique Structure
- Technical Challenges
- Successful Partnership
- Practical Solutions
- Innovative Materials
- Protect the Public Investment



# Questions?

**Chad Robinson, PE**  
*Bridge Inspection  
Program Manager*

**Adrian Lusk, PE**  
*D9 Bridge Engineer*

**David Whited**  
*DOH Project Manager*

**Matt Lewellyn, PE**  
*[Matt.Lewellyn@burgessniple.com](mailto:Matt.Lewellyn@burgessniple.com)*

**Travis Butz, PE**  
*[Travis.Butz@burgessniple.com](mailto:Travis.Butz@burgessniple.com)*



**BURGESS & NIPLE**  
Engineers ■ Architects ■ Planners

